# DEPARTMENT OF TRANSPORTATION

# Assessment of Travel-Time Reliability and Operational Resilience of Metro Freeway Corridors

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Department of Civil Engineering University of Minnesota

December 2024

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# Assessment of Travel-Time Reliability and Operational Resilience of Metro Freeway Corridors

## **Final Report**

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## December 2024

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# **TABLE OF CONTENTS**

Chapter 1: Introduction
1.1 Background and Research Objectives1
1.2 Report Organization1
Chapter 2: DATA COLLECTION AND POPULATION OF TeTRES DATABASE FOR FREEWAY CORRIDORS3
2.1 Introduction
2.2 Collection and Processing of Special Event Data3
2.3 Collection and Processing of Work-Zone Data8
2.4 Collection and Processing of Winter Road-Condition Data12
2.5 Collection and Processing of Incident Data17
2.6 Collection and Processing of Weather Data19
2.7 Summary21
Chapter 3: ESTIMATION AND ANALYSIS OF TRAVEL-TIME TTR MEASURES AND TRENDS FOR THE METRO
FREEWAY CORRIDORS AND NETWORK
3.1 Introduction
3.1 Introduction
3.1 Introduction       22         3.2 Network-wide Traffic-Flow Performance and Travel-Time Reliability Trends       26         3.3 Yearly Trends of Travel-Time Reliability on Individual Directional Routes       32
3.1 Introduction
3.1 Introduction223.2 Network-wide Traffic-Flow Performance and Travel-Time Reliability Trends263.3 Yearly Trends of Travel-Time Reliability on Individual Directional Routes323.4 3.4 Effects of Geometric Configuration on Travel-Time Reliability and Traffic-Flow Measures493.5 Monthly and Yearly Trends of Travel-Time Reliability and Traffic-Flow Measures at Individual
3.1 Introduction223.2 Network-wide Traffic-Flow Performance and Travel-Time Reliability Trends263.3 Yearly Trends of Travel-Time Reliability on Individual Directional Routes323.4 3.4 Effects of Geometric Configuration on Travel-Time Reliability and Traffic-Flow Measures493.5 Monthly and Yearly Trends of Travel-Time Reliability and Traffic-Flow Measures at Individual953.6 Summary103
<b>PREEWAY CORRIDORS AND NETWORK</b> 22         3.1 Introduction       22         3.2 Network-wide Traffic-Flow Performance and Travel-Time Reliability Trends       26         3.3 Yearly Trends of Travel-Time Reliability on Individual Directional Routes       32         3.4 3.4 Effects of Geometric Configuration on Travel-Time Reliability and Traffic-Flow Measures       49         3.5 Monthly and Yearly Trends of Travel-Time Reliability and Traffic-Flow Measures at Individual       95         3.6 Summary       103         Chapter 4: ENHANCEMENT OF THE OPERATIONAL-RESILIENCE MODEL FOR FREEWAY CORRIDORS104
<b>PREEWAY CORRIDORS AND NETWORK</b> 22         3.1 Introduction       22         3.2 Network-wide Traffic-Flow Performance and Travel-Time Reliability Trends       26         3.3 Yearly Trends of Travel-Time Reliability on Individual Directional Routes       32         3.4 3.4 Effects of Geometric Configuration on Travel-Time Reliability and Traffic-Flow Measures       49         3.5 Monthly and Yearly Trends of Travel-Time Reliability and Traffic-Flow Measures at Individual       95         3.6 Summary       103 <b>Chapter 4: ENHANCEMENT OF THE OPERATIONAL-RESILIENCE MODEL FOR FREEWAY CORRIDORS104</b> 4.1 Introduction and Overview of Preliminary Resilience Model       104
<b>PREEWAY CORRIDORS AND NETWORK</b> 22         3.1 Introduction       22         3.2 Network-wide Traffic-Flow Performance and Travel-Time Reliability Trends       26         3.3 Yearly Trends of Travel-Time Reliability on Individual Directional Routes       32         3.4 3.4 Effects of Geometric Configuration on Travel-Time Reliability and Traffic-Flow Measures       49         3.5 Monthly and Yearly Trends of Travel-Time Reliability and Traffic-Flow Measures at Individual       95         3.6 Summary       103 <b>Chapter 4: ENHANCEMENT OF THE OPERATIONAL-RESILIENCE MODEL FOR FREEWAY CORRIDORS104</b> 4.1 Introduction and Overview of Preliminary Resilience Model       104         4.2 Enhancements of the Preliminary Model for Operational Resilience of Freeway Corridors       106

Chapter 5: ASSESSMENT OF OPERATIONAL RESILIENCE OF INDIVIDUAL CORRIDORS AND THEIR EFFECTS ON TRAFFIC-FLOW PERFORMANCE IN THE METRO NETWORK
5.1 Introduction115
5.2 Estimation of Operational Resilience of Individual Directional Corridors in the Metro Network 115
5.3 Effects of Operational Resilience on Travel-Time Reliability and Traffic-Flow Measures136
5.4 Summary
Chapter 6: CONCLUSIONS – RESEARCH BENEFITS/IMPLEMENTATION/FUTURE STUDY NEEDS
REFERENCES
APPENDIX A: Monthly and Yearly Estimates of Travel-Time Reliability and Traffic-Flow Measures for Individual Directional Corridors in the Metro Freeway Network (1/2018 – 12/2023)

## **LIST OF FIGURES**

Figure 2.2.1 A Sample Organized Data Set for the Special Events at Huntington Bank Stadium5
Figure 2.2.2 A Sample Organized Dataset for the Non-Sport Events at Target Center
Figure 2.2.3 A Screenshot of the Script for Populating T-database with the Special-Event Data7
Figure 2.2.4 A Screenshot of the T-database Showing a Portion of Populated Special-Event Data7
Figure 2.3.1 Construction-Project Maps in the Metro Freeway Network (Source: Metro District, MnDOT) 8
Figure 2.3.2 A Sample Screenshot of the Detailed Information for the Metro Construction Projects9
Figure 2.3.3 An Example Screenshot of the Excel-formatted Work-Zone Data File
Figure 2.3.4 The Snippets of the Script for Processing Work-Zone Data
Figure 2.3.5 A Screenshot of the T-database Showing a Portion of the Work-Zone Data
Figure 2.4.1 A Portion of the Snow-Event Data for the Metro Freeways (Source: Metro District, MnDOT)13
Figure 2.4.2 A portion of the Metro-Freeway Snow-Plow Route Location Data with Station IDs
Figure 2.4.3 A Sample Snow-Event Data for 2018-19 in Excel Format Processed for T-database15
Figure 2.4.4 The Snippets of the Script for Populating T-database with Snow-Event Data
Figure 2.4.5 Screenshot of T-database with a Portion of Populated Snow-Event Data
Figure 2.5.1 Structure of the Incident Data Reader/T-Database Population Module
Figure 2.5.2 Snippets of the Script for Populating T-database with Incident data
Figure 2.5.3 Format Changes in CAD and IRIS Incident Databases19
Figure 2.5.4 A Screenshot of T-database Showing a Portion of Populated Incident Data
Figure 2.6.1 The Snippets of Script for Downloading Weather Data and Populating T-Database
Figure 2.6.2 A screenshot of T-Database Showing a Portion of Populated Weather Data
Figure 3.1.1 Freeway Corridors defined for TTR and Traffic-Flow MoE Estimation
Figure 3.2.1 Network-wide Monthly Variations of Traffic-Flow Measures of Effectiveness
Figure 3.2.2 Network-wide Yearly Variations of Traffic-Flow MoEs for Morning and Afternoon Routes28

Figure 3.2.3 Monthly Network-wide TTR Trends for Morning and Afternoon Routes	30
Figure 3.2.4 Yearly Network-wide TTR Trends for Morning and Afternoon Routes	31
Figure 3.3.1 Vulnerability Level Definitions in a BI-TR Space	33
Figure 3.3.2 BI-TR Relationship - 2018 Morning Routes	33
Figure 3.3.3 BI-TR Relationship - 2019 Morning Routes	34
Figure 3.3.4 BI-TR Relationship - 2020 Morning Routes	35
Figure 3.3.5 BI-TR Relationship - 2021 Morning Routes	36
Figure 3.3.6 BI-TR Relationship - 2022 Morning Routes	36
Figure 3.3.7 BI-TR Relationship - 2023 Morning Routes	37
Figure 3.3.8 BI-TR Relationships – 2018 Afternoon Routes	38
Figure 3.3.9 BI-TR Relationships – 2019 Afternoon Routes	39
Figure 3.3.10 BI-TR Relationships – 2020 Afternoon Routes	40
Figure 3.3.11 BI-TR Relationships – 2021 Afternoon Routes	41
Figure 3.3.12 BI-TR Relationships – 2022 Afternoon Routes	41
Figure 3.3.13 BI-TR Relationships - 2023 Afternoon Routes	42
Figure 3.3.14 Yearly Variations of Vulnerability Levels (Morning/Afternoon Routes)	49
Figure 3.4.1 BI – G Relationship (2018 Morning Routes)	51
Figure 3.4.2 BI – G Relationship (2019 Morning Routes)	52
Figure 3.4.3 BI – G Relationship (2020 Morning Routes)	53
Figure 3.4.4 BI – G Relationship (2021 Morning Routes)	54
Figure 3.4.5 BI – G Relationship (2022 Morning Routes)	55
Figure 3.4.6 BI – G Relationship (2023 Morning Routes)	56
Figure 3.4.7 BI – G Relationship (2018 Afternoon Routes)	57
Figure 3.4.8 BI – G Relationship (2019 Afternoon Routes)	57

Figure 3.4.9 BI – G Relationship (2020 Afternoon Routes)	58
Figure 3.4.10 BI – G Relationship (2021 Afternoon Routes)	59
Figure 3.4.11 BI – G Relationship (2022 Afternoon Routes)	60
Figure 3.4.12 BI - G Relationship (2023 Afternoon Routes)	61
Figure 3.4.13 PI - G Relationship (2018 Morning Routes)	62
Figure 3.4.14 PI - G Relationship (2019 Morning Routes)	62
Figure 3.4.15 PI - G Relationship (2020 Morning Routes)	63
Figure 3.4.16 PI - G Relationship (2021 Morning Routes)	64
Figure 3.4.17 PI - G Relationship (2022 Morning Routes)	65
Figure 3.4.18 PI - G Relationship (2023 Morning Routes)	66
Figure 3.4.19 PI - G Relationship (2018 Afternoon Routes)	67
Figure 3.4.20 PI - G Relationship (2019 Afternoon Routes)	68
Figure 3.4.21 PI - G Relationship (2020 Afternoon Routes)	68
Figure 3.4.22 PI - G Relationship (2021 Afternoon Routes)	69
Figure 3.4.23 PI - G Relationship (2022 Afternoon Routes)	70
Figure 3.4.24 PI - G Relationship (2023 Afternoon Routes)	71
Figure 3.4.25 VMT/mile - G Relationship (2018 Morning Routes)	72
Figure 3.4.26 VMT/mile - G Relationship (2019 Morning Routes)	73
Figure 3.4.27 VMT/mile - G Relationship (2020 Morning Routes)	74
Figure 3.4.28 VMT/mile - G Relationship (2021 Morning Routes)	75
Figure 3.4.29 VMT/mile - G Relationship (2022 Morning Routes)	76
Figure 3.4.30 VMT/mile - G Relationship (2023 Morning Routes)	77
Figure 3.4.31 VMT/mile - G Relationship (2018 Afternoon Routes)	78
Figure 3.4.32 VMT/mile - G Relationship (2019 Afternoon Routes)	

Figure 3.4.33 VMT/mile - G Relationship (2020 Afternoon Routes)	. 80
Figure 3.4.34 VMT/mile - G Relationship (2021 Afternoon Routes)	.81
Figure 3.4.35 VMT/mile - G Relationship (2022 Afternoon Routes)	. 82
Figure 3.4.36 VMT/mile - G Relationship (2023 Afternoon Routes)	. 83
Figure 3.4.37 VHT/mile - G Relationship (2018 Morning Routes)	. 84
Figure 3.4.38 VHT/mile - G Relationship (2019 Morning Routes)	. 85
Figure 3.4.39 VHT/mile - G Relationship (2020 Morning Routes)	. 86
Figure 3.4.40 VHT/mile - G Relationship (2021 Morning Routes)	. 87
Figure 3.4.41 VHT/mile - G Relationship (2022 Morning Routes)	. 88
Figure 3.4.42 VHT/mile - G Relationship (2023 Morning Routes)	. 89
Figure 3.4.43 VHT/mile - G Relationship (2018 Afternoon Routes)	. 90
Figure 3.4.44 VHT/mile - G Relationship (2019 Afternoon Routes)	.91
Figure 3.4.45 VHT/mile - G Relationship (2020 Afternoon Routes)	.92
Figure 3.4.46 VHT/mile - G Relationship (2021 Afternoon Routes)	.93
Figure 3.4.47 VHT/mile - G Relationship (2022 Afternoon Routes)	.94
Figure 3.4.48 VHT/mile - G Relationship (2023 Afternoon Routes)	. 95
Figure 3.5.1 Location of 169 NB Route	.96
Figure 3.5.2 Monthly Variations of TTR Measures under Different Weather Conditions (169 NB)	.96
Figure 3.5.3 Monthly Variations of TTR Measures under Different Incident Conditions (169 NB)	.97
Figure 3.5.4 Monthly Variations of TTR Measures under Different Work-Zone Conditions (169 NB)	.98
Figure 3.5.5 Yearly Variations of TTR Measures under Different Weather Conditions (169 NB)	. 99
Figure 3.5.6 Yearly Variations of TTR Measures under Different Incident Conditions (169 NB)	100
Figure 3.5.7 Yearly Variations of TTR Measures under Different Work-Zone Conditions (169 NB)	100
Figure 3.5.8 Yearly Variations of Buffer-Index vs Travel-Rate Relationships (169 NB)	101

Figure 3.5.9 Yearly Variations of Traffic-Flow Measures (169 NB)10	2
Figure 4.1.1 Conceptual Framework of Corridor-wide Traffic Process	4
Figure 4.1.2 Locations of the Sample Routes (5 Corridors/8 Directional Routes)	5
Figure 4.1.3 Operational Resilience (Preliminary Model) vs G for Sample Routes	6
Figure 4.2.1 Space-Mean-Speed, Ut, Variations through time at I-494 NB and 100 NB Routes	7
Figure 4.2.2 Vt and Ut Variations through time at I-494 NB Route10	8
Figure 4.2.3 Comparison of Daily Estimates of CORI from M1 and M2 (Afternoon-Peak Routes)	0
Figure 4.2.4 Comparison of Daily Estimates of CORI from M 1 and M 2 (Morning-Peak Routes)11	1
Figure 4.2.5 Comparison of Daily CORI Estimates with Preliminary Model and Enhanced Model (M2) 11	2
Figure 4.2.6 CORI vs Geometric Friction Index (G) with Model 1 and Model 211	3
Figure 4.2.7 Comparison of Dry-Day and Rainy-Day CORI Estimates with Model 2	3
Figure 5.2.1 Configuration of Individual Corridors for the Metro Freeway Network	6
Figure 5.2.2 CORI Levels (2018, AM, Dry Days)11	9
Figure 5.2.3 CORI Levels (2018, PM, Dry Days)12	1
Figure 5.2.4 CORI Levels (2019, AM, Dry Days)12	3
Figure 5.2.5 CORI Levels (2019, PM, Dry Days)12	5
Figure 5.2.6 CORI Levels (2023, AM, Dry Days)12	7
Figure 5.2.7 CORI Levels (2023, PM, Dry Days)12	9
Figure 5.2.8 Route-wide Resilience vs. Geometric Friction Factor (2018, Dry Days, AM-Peak Routes) 13	1
Figure 5.2.9 Route-wide Resilience vs. Geometric Friction Factor (2018, Dry Days, PM-Peak Routes)13	2
Figure 5.2.10 Route-wide Resilience vs. Geometric Friction Factor (2019, Dry Days, AM-Peak Routes) 13	3
Figure 5.2.11 Route-wide Resilience vs. Geometric Friction Factor (2019, Dry Days, PM-Peak Routes)13	4
Figure 5.2.12 Route-wide Resilience vs. Geometric Friction Factor (2023, Dry Days, AM-Peak Routes) 13	5
Figure 5.2.13 Route-wide Resilience vs. Geometric Friction Factor (2023, Dry Days, PM-Peak Routes)13	6

Figure 5.3.1 Route-wide Resilience Index vs VMT/mile (2018, Morning-Peak Routes)	138
Figure 5.3.2 Route-wide Resilience Index vs VMT/mile (2018, Afternoon-Peak Routes)	139
Figure 5.3.3 Route-wide Resilience Index vs VMT/mile (2019, Morning-Peak Routes)	140
Figure 5.3.4 Route-wide Resilience Index vs VMT/mile (2019, Afternoon-Peak Routes)	141
Figure 5.3.5 Route-wide Resilience Index vs VMT/mile (2023, Morning-Peak Routes)	142
Figure 5.3.6 Route-wide Resilience Index vs VMT/mile (2023, Afternoon-Peak Routes)	143
Figure 5.3.7 Route-wide Resilience Index vs DVH/mile (2018, Morning-Peak Routes)	144
Figure 5.3.8 Route-wide Resilience Index vs DVH/mile (2018, Afternoon-Peak Routes)	145
Figure 5.3.9 Route-wide Resilience Index vs DVH/mile (2019, Morning-Peak Routes)	146
Figure 5.3.10 Route-wide Resilience Index vs DVH/mile (2019, Afternoon-Peak Routes)	147
Figure 5.3.11 Route-wide Resilience Index vs DVH/mile (2023, Morning-Peak Routes)	148
Figure 5.3.12 Route-wide Resilience Index vs DVH/mile (2023, Afternoon-Peak Routes)	149
Figure 5.3.13 Route-wide Resilience Index vs Buffer Index (2018, Morning-Peak Routes)	150
Figure 5.3.14 Route-wide Resilience Index vs Buffer Index (2018, Afternoon-Peak Routes)	151
Figure 5.3.15 Route-wide Resilience Index vs Buffer Index (2019, Morning-Peak Routes)	152
Figure 5.3.16 Route-wide Resilience Index vs Buffer Index (2019, Afternoon-Peak Routes	153
Figure 5.3.17 Route-wide Resilience Index vs Buffer Index (2023, Morning-Peak Routes)	154
Figure 5.3.18 Route-wide Resilience Index vs Buffer Index (2023, Afternoon-Peak Routes)	155
Figure 5.3.19 Route-wide Resilience Index vs Planning Index (2018, Morning-Peak Routes)	156
Figure 5.3.20 Route-wide Resilience Index vs Planning Index (2018, Afternoon-Peak Routes)	157
Figure 5.3.21 Route-wide Resilience Index vs Planning Index (2019, Morning-Peak Routes)	158
Figure 5.3.22 Route-wide Resilience Index vs Planning Index (2019, Afternoon-Peak Routes)	159
Figure 5.3.23 Route-wide Resilience Index vs Planning Index (2023, Morning-Peak Routes)	160
Figure 5.3.24 Route-wide Resilience Index vs Planning Index (2023, Afternoon-Peak Routes)	161

## LIST OF TABLES

Table 3.1.1 Start/End Stations of Each Corridor Directional Route
Table 3.3.1 Yearly BI-TR and VI Levels for Morning Routes (2018-2020)       43
Table 3.3.2 Yearly BI-TR and VI Levels for Morning Routes (2021-2023)
Table 3.3.3 Yearly BI-TR and VI Levels for Afternoon Routes (2018-2020)46
Table 3.3.4 Yearly BI-TR and VI Levels for Afternoon Routes (2021-2023)47
Table 3.3.5 Yearly Variations of Vulnerability Levels for Morning and Afternoon Routes
Table 5.2.1 CORI Values (2018, AM, Dry Days)117
Table 5.2.2 CORI Values (2018, PM, Dry Days)119
Table 5.2.3 CORI Levels (2018, PM, Dry Days)121
Table 5.2.4 CORI Values (2019, PM, Dry Days)124
Table 5.2.5 CORI Values (2023, AM, Dry Days)125
Table 5.2.6 CORI Values (2023, PM, Dry Days)127
Table 5.2.7 Average CORI Values of Individual Corridors with Level 4 or Higher (Morning-Peak Period). 129
Table 5.2.8 Average CORI Values of Individual Corridors with Level 4 or Higher (Afternoon-Peak Period)

## **EXECUTIVE SUMMARY**

A key element in developing and maintaining a reliable and resilient freeway network is the ability to monitor and assess the travel-time reliability (TTR) and resilience at individual corridor levels under various operating conditions. The previous phases of this research developed and enhanced the Travel-Time Reliability Estimation System (TeTRES), which had been applied to estimate the TTR and traffic-flow measures of the 48 directional corridors in the metro freeway network from January 2016 to September 2020. In the current study, the metro-freeway network was reconfigured to have a total of 74 directional corridors, whose TTR and traffic-flow measures were continuously estimated until December 2023. The results from the individual-route analysis were used to determine the network-wide trends for the TTR and traffic-flow measures before and after April 2020, when the COVID-19 traffic-restriction started. The trends analysis showed that, after the sudden reduction in traffic flows in April 2020, the traffic flows in the metro freeway network have been slowly but continuously increasing; however, as of December 2023, they had not reached the pre-pandemic level. As a result, the network-wide TTR measures after April 2020 showed continuously better reliability conditions than those of the pre-pandemic period in both morning and afternoon networks. Furthermore, the yearly level of route vulnerability, i.e., the overall reliability condition of each route combining the buffer and planning indices, was also determined, and the most vulnerable routes were identified for each year from 2018 until 2023. In addition, the effects of the route-wide geometric configuration, quantified with the geometric-friction index (G), on the TTR and traffic-flow measures of each route were also analyzed by examining the G-TTR/traffic-flow measure relationships, which exhibited the expected patterns, i.e., the routes with low geometric friction handled higher traffic flows with less fluctuations in travel times than those with high friction.

This study also enhanced the preliminary resilience model, developed in the previous phase, by adopting the route-wide space-mean speed as the measure of the system performance. The resulting enhanced model showed significantly better performance than those from the preliminary model in terms of the consistency in the daily estimates of the corridor-wide operational resilience (CORI) for the sample routes. Furthermore, the CORI-geometric friction index (G) relationships for the sample routes show clear patterns with strong correlations for both dry and rainy periods. The enhanced model was applied to estimate the CORI of 74 directional corridors and a group of the directional routes exhibiting consistently low-level of resilience were identified for both morning and afternoon-peak periods. The CORI-G relationships of the metro corridors indicated that the routes with efficient geometric structure in terms of handling through traffic showed strong resilience compared with those routes with high levels of geometric friction. Finally, the analysis of the relationships between CORI and the TTR/traffic-flow measures at directional corridors indicated that the routes with strong resilience also showed better productivity and reliability, e.g., higher VMT/mile with less variability in travel times, than those routes with low levels of resilience.

The results from this research could provide the basis for geometric and operational improvements of the metro freeway corridors. Future research needs include the continuous assessment of the TTR/traffic-flow measures for the metro freeway network, the enhancement of the geometry-friction model by incorporating the connectivity and accessibility of a given route to adjacent corridors, and the study on the potential effects of heavy-vehicle flows on the corridor-wide operational resilience in the metro freeway network.

## **Chapter 1: Introduction**

#### **1.1 Background and Research Objectives**

A key element in developing and maintaining a reliable and resilient freeway network is the capability to monitor and assess the travel-time reliability and resilience at individual corridor levels under various operating conditions. The previous phases of this research developed and enhanced the Travel-Time Reliability Estimation System (TeTRES), which has been applied to estimate and analyze the trends of the travel-time reliability and traffic-flow measures of the 48 directional corridors in the metro freeway network under different operating conditions from January 2016 to September 2020 [1, 2]. Furthermore, a preliminary model was developed to estimate the corridor-wide operational-resilience index (CORI), designed to quantify the inherent capability of a given corridor system in resisting and recovering from congestion. The prototype CORI model was tested with the sample data collected from 6 directional corridors in the metro area under dry weather conditions.

This study continuously assesses the reliability trends of the metro freeway network with TeTRES. Specifically, the travel-time reliability (TTR) measures of the individual freeway corridors are estimated until December 2023 and their historical trends under various operating conditions are analyzed. Furthermore, the prototype resilience model, developed from the previous phase, is enhanced to reflect different weather conditions with an expanded data set, and applied to determine the operational resilience of the individual corridors in the metro network. The specific objectives of this study include:

- Collection and processing of historical traffic and non-traffic data, required for travel-time reliability estimation, from multiple sources until December 2023, and population of the TeTRES database with the updated dataset.
- Estimation of monthly and yearly TTR measures for the individual corridors in the metro freeway network under various operating conditions from January 2018 to December 2023.
- Enhancement of the preliminary resilience model and estimation of the operational resilience of the individual corridors in the metro freeway network.
- Assessment of the effects of operational resilience on traffic-flow performance at individual corridors.

#### **1.2 Report Organization**

Chapter 2 describes the process to collect the historical traffic and non-traffic data and to populate the TeTRES database, where the travel times of a new set of directional freeway corridors are calculated, stored and linked to various operating conditions until December 2023. In Chapter 3, the updated TeTRES database is applied to estimate the travel-time reliability and traffic-flow measures for the individual freeway corridors in the metro network. Chapter 4 enhances the preliminary model to quantify the operational resilience of individual directional corridors. Using the enhanced resilience model, the

operational resilience of individual directional corridors are estimated and assessed in Chapter 5. The effects of the operational resilience on the traffic-flow performance at individual corridors are also analyzed in Chapter 5. Finally, Chapter 6 summarizes conclusions, the benefits of the current study, potential implementation steps and future study needs.

# Chapter 2: DATA COLLECTION AND POPULATION OF TeTRES DATABASE FOR FREEWAY CORRIDORS

## 2.1 Introduction

This chapter summarizes the types of data collected and processed in this study to continuously estimate and assess the travel-time reliability and traffic-flow measures for the freeway corridors in the metro network. The following list shows the types and time periods of the data collected and processed in the previous phase of this study [2]:

- Special- event data, including for both sport and non-sport events, from the venues located in the Twin Cities' metro area (1/2012-3/2020)
- Construction project location and time-period data on the metro freeways (4/2012 9/2020)
- Winter road-surface condition data for the metro freeways (10/2012 4/2019)
- Incident data from CAD, Computer-Aided-Dispatch system, Department of Public Safety, and IRIS, Intelligent Roadway Information System, MnDOT (1/2010 – 9/30/2020)
- Weather data from NOAA, National Oceanic and Atmospheric Administration (NOAA: 1/2010 9/30/2020)
- Traffic-detector data from the metro freeways (stored separately in a local hard disk): (1/2010 9/2020).

In the current study, the same types of data in the above list were collected for the period from the end of the previous phase until December 2023. It can be noted that the traffic-detector data, necessary for calculating route-travel times, are separately stored in a local hard disk in a structured format, while all other types of non-traffic data are processed separately and stored in the TeTRES database (T-database) following the schema and table formats developed in the previous study [1]. Further, the weather data from NOAA has been automatically downloaded and stored in the T-database by the weather-data processing module in TeTRES, while all other types of non-traffic data are manually processed to a set of the Excel-formatted files, which are then batch-processed to populate the T-database using a set of the scripts specifically developed for each data type. In particular, the data for the non-sport events, e.g., concerts, were collected manually from the publicly available websites of each concert.

The rest of this chapter describes the main features of each type of data collected and processed in this study. The updated T-database populated with all the new data collected in this study is applied to estimate the travel-time reliability and traffic-flow measures of the metro freeway corridors in the next chapter.

#### 2.2 Collection and Processing of Special Event Data

First, the data for the Twin Cities' metro-area special events, whose attendance can affect the traffic conditions on the nearby freeways, are collected manually using the publicly available data sources for

the period of March 2020 until December 2023. Specifically, the data for both sport and non-sport events, such as music concerts, were collected from the relevant websites for each event. The collected data for each event includes event name, date/start/end times, attendance, and coordinates of an event location. The following list shows the names of the special event locations, and the sources of the sportevent data collected for each location:

• U.S. Bank Stadium:

Main Events: National Football League Games

Data Sources: <a href="https://www.pro-football-reference.com/boxscores/">https://www.pro-football-reference.com/boxscores/</a>

https://www.espn.com/nfl/team/schedule/ /name/min/

• Target Center:

Main Events: National Basketball Association Games

- Data Sources: https://www.basketball-reference.com/teams/MIN/
- Target Field:

Main Events: Major League Baseball Games

Data Sources: https://www.baseball-reference.com/teams/MIN/

https://www.espn.com/mlb/team/schedule/\_/name/min/

• Huntington Bank Stadium:

Main Events: College Football Games

Data Sources: <a href="https://www.espn.com/nfl/team/schedule/\_/name/min/">https://www.espn.com/nfl/team/schedule/\_/name/min/</a>

https://gophersports.com/sports/football/schedule/2022?path=football

• Xcel Energy Center:

Event: National Hockey League Games

Data Sources: https://www.hockey-reference.com/leagues/

https://www.nhl.com/gamecenter/

https://www.espn.com/nhl/team/schedule/\_/name/min/season/2019

Event: Minnesota State Highschool League Hockey Games

Data Source: https://www.mshsl.org/tournaments/state-tournament-archives/state-

#### tournament-archive-boys-hockey-2022

For the non-sport event data, e.g., music concerts, due to the lack of relevant data from the agencies managing those venues, the data for each event needed to be searched and collected manually from publicly available websites. For example, the following websites were identified by searching the internet and used to collect the attendance data for two concert events at Target Center, i.e., 'Mercury Tour' and 'Metallica-Worldwired Tour':

https://en.wikipedia.org/wiki/Mercury\_World\_Tour

#### https://touringdata.wordpress.com/2019/07/18/metallica-worldwired-tour/

Figure 2.2.1 shows a portion of the Special-Event data collected and organized in an Excel format for the 2020-2022 events at the Huntington Bank Stadium in this task. A portion of the non-sport event data from the Target Center is also shown in Figure 2.2.2.

The special-event data collected and organized in an Excel format are then uploaded to the T-database using the script specifically designed to populate the T-database with the special-event data. Figures 2.2.3 and 2.2.4 show the screenshot of the script and a portion of the T-database populated with the special-event data collected in this study. The resulting T-database contains the sport-event data in the Twin Cities' metro area for the period of 1/2012 - 12/2023, while the non-sport event data ranges from 2016 to 2023.

DATE	START	END	TITLE	TYPE	ATTEND	LAT	LON
2020-10-24	18:30:00	22:11:00	MN Gophers vs Michigan	Football	589	44.9765288	-93.2267402
2020-11-13	18:05:00	21:20:00	MN Gophers vs Iowa	Football	771	44.9765288	-93.2267402
2020-11-20	18:32:00	21:56:00	MN Gophers vs Purdue	Football	593	44.9765288	-93.2267402
2021-09-02	19:05:00	22:35:00	MN Gophers vs Ohio State	Football	50805	44.9765288	-93.2267402
<u>2021-09-11</u>	11:05:00	14:28:00	MN Gophers vs Miami (Ohio)	Football	43372	44.9765288	-93.2267402
2021-09-25	11:07:00	14:24:00	MN Gophers vs Bowling Green	Football	46236	44.9765288	-93.2267402
<u>2021-10-16</u>	11:01:00	14:28:00	MN Gophers vs Nebraska	Football	45436	44.9765288	-93.2267402
2021-10-23	14:35:00	17:47:00	MN Gophers vs Maryland	Football	41011	44.9765288	-93.2267402
<u>2021-11-06</u>	11:01:00	14:13:00	MN Gophers vs Illinois	Football	46382	44.9765288	-93.2267402
2021-11-27	15:10:00	18:41:00	MN Gophers vs Wisconsin	Football	49736	44.9765288	-93.2267402
<u>2022-09-01</u>	20:05:00	23:08:00	MN Gophers vs New Mexico State	Football	44012	44.9765288	-93.2267402
2022-09-10	11:01:00	14:25:00	MN Gophers vs Western illinois	Football	43859	44.9765288	-93.2267402
<u>2022-09-17</u>	14:35:00	17:55:00	MN Gophers vs Colorado	Football	42101	44.9765288	-93.2267402
2022-10-01	11:01:00	14:22:00	MN Gophers vs Purdue	Football	48288	44.9765288	-93.2267402

Figure 2.2.1 A Sample Organized Data Set for the Special Events at Huntington Bank Stadium

DATE	START	END	TITLE	TYPE	ATTEND	LAT	LON
2022-12-05	21:15:00	22:35:00	Five Finger Death Punch at Target Center	Concert	13110	44.98	-93.28
2022-12-05	19:30:00	20:40:00	Brantley Gilbert at Target Center	Concert	13110	44.98	-93.28
2022-12-05	18:30:00	19:00:00	Cory Marks at Target Center	Concert	13110	44.98	-93.28
2022-11-13	19:30:00	23:20:00	Dave Matthews Band	Concert	9821	44.98	-93.28
2022-11-11	20:00:00	22:45:00	Rod Wave Beautiful Mind Tour	Concert	13110	44.98	-93.28
2022-10-25	19:30:00	22:40:00	Denim & Rhinestones Tour	Concert	13110	44.98	-93.28
2022-10-07	20:00:00	23:00:00	Bill Burr	Concert	13110	44.98	-93.28
2022-09-22	20:00:00	22:30:00	Ben Platt	Concert	13110	44.98	-93.28
2022-08-26	19:00:00	23:00:00	Kevin Hart	Concert	13110	44.98	-93.28
2022-07-30	20:00:00	23:05:00	Roger Waters - This is not a Drill	Concert	10913	44.98	-93.28
2022-06-24	20:00:00	23:00:00	keith sweat	Concert	13110	44.98	-93.28
2022-06-13	19:00:00	21:00:00	Chris Tomlin and Hillsong United at Target Center	Concert	13110	44.98	-93.28
2022-05-07	17:45:00	23:05:00	Twin city Takeover	Concert	13110	44.98	-93.28
2022-05-06	19:15:00	22:30:00	Justin Bieber, Jaden and Eddie Benjamin	Concert	13192	44.98	-93.28
2022-04-25	19:00:00	22:35:00	MercyMe and Rend Collective	Concert	13110	44.98	-93.28
2022-04-09	18:30:00	23:00:00	Slipknot at Target Center	Concert	13110	44.98	-93.28
2022-03-26	18:30:00	23:00:00	Korn, Chevelle and Code Orange	Concert	13110	44.98	-93.28
2022-03-20	15:00:00	18:00:00	Harlem Globetrotters	Event	13110	44.98	-93.28
2022-03-15	19:30:00	23:00:00	Happier Than Ever The World Tour	Concert	14924	44.98	-93.28
2022-03-13	19:30:00	23:00:00	Tool ,The Acid Helps	Concert	13110	44.98	-93.28
2022-03-12	19:00:00	23:00:00	Hits Deep Tour	Concert	13110	44.98	-93.28
2022-03-08	19:30:00	23:00:00	Dua Lipa Tour	Concert	11987	44.98	-93.28
2022-02-27	19:00:00	23:00:00	Mercury Tour	Concert	12587	44.98	-93.28
2022-02-26	19:00:00	23:00:00	Chris Tomlin Hillsong United at Target Center	Concert	13110	44.98	-93.28
2022-02-20	19:00:00	23:00:00	Call Me If You Get Lost Tour	Concert	9691	44.98	-93.28
2022-02-05	19:00:00	23:00:00	Volbeat, Ghost and Twin Temple	Concert	13110	44.98	-93.28

Figure 2.2.2 A Sample Organized Dataset for the Non-Sport Events at Target Center



Figure 2.2.3 A Screenshot of the Script for Populating T-database with the Special-Event Data

ß	🔗 public.specialevent/tetres/postgres@demo 🕏										
Quer	Query Query History Scratch Pad ×										
1	1 SELECT * FROM public.specialevent										
2	2 ORDER BY 1d DESC LIMIT 100										
Data	Data output Messages Notifications										
=+											
	id [PK] integer	name character varying (100)	description /	years character varying (255)	start_time timestamp without time zone	end_time timestamp without time zone	double precision	lon double precision /	attendance /	reg_date timestamp without time zone	
1	5640	Minnesota Wild vs St	Hockey	2022	2022-05-10 21:30:00	2022-05-10 23:59:00	44.9448353	-93.103291	19197	2022-10-10 11:41:17.073083	
2	5639	Minnesota Wild vs St	Hockey	2022	2022-05-04 21:30:00	2022-05-04 23:59:00	44.9448353	-93.103291	19376	2022-10-10 11:41:15.010725	
3	5638	Minnesota Wild vs St	Hockey	2022	2022-05-02 21:30:00	2022-05-02 23:59:00	44.9448353	-93.103291	19053	2022-10-10 11:41:12.963961	
4	5637	Minnesota Wild vs Col	Hockey	2022	2022-04-29 20:00:00	2022-04-29 22:30:00	44.9448353	-93.103291	19261	2022-10-10 11:41:10.917259	
5	5636	Minnesota Wild vs Cal	Hockey	2022	2022-04-28 20:00:00	2022-04-28 22:25:00	44.9448353	-93.103291	18490	2022-10-10 11:41:08.854902	
6	5635	Minnesota Wild vs Ariz	Hockey	2022	2022-04-26 20:00:00	2022-04-26 22:30:00	44.9448353	-93.103291	18383	2022-10-10 11:41:06.792544	
7	5634	Minneosta Wild vs Sea	Hockey	2022	2022-04-22 20:00:00	2022-04-22 22:30:00	44.9448353	-93.103291	19047	2022-10-10 11:41:04.745811	
8	5633	Minnesota Wild vs Van	Hockey	2022	2022-04-21 20:00:00	2022-04-21 22:30:00	44.9448353	-93.103291	17894	2022-10-10 11:41:02.683454	
9	5632	Minnesota Wild vs San	Hockey	2022	2022-04-17 18:00:00	2022-04-17 20:30:00	44.9448353	-93.103291	19029	2022-10-10 11:41:00.621097	
10	5631	Minnesota Wild vs Ed	Hockey	2022	2022-04-12 20:00:00	2022-04-12 22:30:00	44.9448353	-93.103291	19035	2022-10-10 11:40:58.558739	
11	5630	Minnesota Wild vs Los	Hockey	2022	2022-04-10 17:00:00	2022-04-10 19:30:00	44.9448353	-93.103291	19104	2022-10-10 11:40:56.44951	
12	5629	Minnesota Wild vs Pitt	Hockey	2022	2022-03-31 20:00:00	2022-03-31 22:40:00	44.9448353	-93.103291	18978	2022-10-10 11:40:54.387152	
13	5628	Minneosta Wild vs Phil	Hockey	2022	2022-03-29 20:00:00	2022-03-29 22:25:00	44.9448353	-93.103291	17874	2022-10-10 11:40:52.324795	
14	5627	Minnesota Wild vs Col	Hockey	2022	2022-03-27 18:00:00	2022-03-27 20:40:00	44.9448353	-93.103291	19140	2022-10-10 11:40:50.278062	
15	5626	Minnesota Wild vs Col	Hockey	2022	2022-03-26 20:00:00	2022-03-26 22:40:00	44.9448353	-93.103291	19089	2022-10-10 11:40:48.231329	
16	5625	Minnesota Wild vs Van	Hockey	2022	2022-03-24 20:00:00	2022-03-24 22:40:00	44.9448353	-93.103291	17333	2022-10-10 11:40:45.76275	
17	5624	Minnesota Wild vs Veg	Hockey	2022	2022-03-21 20:00:00	2022-03-21 22:20:00	44.9448353	-93.103291	17498	2022-10-10 11:40:43.73164	
18	5623	Minnesota Wild vs Chi	Hockey	2022	2022-03-19 14:00:00	2022-03-19 16:30:00	44.9448353	-93.103291	19226	2022-10-10 11:40:41.684907	
19	5622	Minnesota Wild vs Bos	Hockey	2022	2022-03-16 19:30:00	2022-03-16 22:10:00	44.9448353	-93.103291	17956	2022-10-10 11:40:39.622549	
20	5621	Minnesota Wild vs Nas	Hockey	2022	2022-03-13 19:00:00	2022-03-13 21:30:00	44.9448353	-93.103291	19009	2022-10-10 11:40:37.560192	
21	5620	Minneosta Wild vs Ne	Hockey	2022	2022-03-08 20:00:00	2022-03-08 22:20:00	44.9448353	-93.103291	18356	2022-10-10 11:40:35.497834	
22	5619	Minnesota Wild vs Dall	Hockey	2022	2022-03-06 16:00:00	2022-03-06 18:40:00	44.9448353	-93.103291	18791	2022-10-10 11:40:33.435478	
23	5618	Minnesota Wild vs Cal	Hockey	2022	2022-03-01 20:00:00	2022-03-01 22:30:00	44.9448353	-93.103291	16998	2022-10-10 11:40:31.373121	
24	5617	Minnesota Wild vs Flor	Hockey	2022	2022-02-18 20:00:00	2022-02-18 22:30:00	44.9448353	-93.103291	18300	2022-10-10 11:40:29.310763	
25	5616	Minnesota Wild vs Detr	Hockey	2022	2022-02-14 20:00:00	2022-02-14 22:40:00	44.9448353	-93.103291	18098	2022-10-10.11:40:27.248405	
26	5615	Minnesota Wild vs Car	Hockey	2022	2022-02-12 20:00:00	2022-02-12 22:30:00	44.9448353	-93.103291	18802	2022-10-10-11:40:25.076681activate	

Figure 2.2.4 A Screenshot of the T-database Showing a Portion of Populated Special-Event Data

#### 2.3 Collection and Processing of Work-Zone Data

Figures 2.3.1 and 2.3.2 show the construction-project maps and a sample portion of the detailed information for each project, provided by the Metro District, MnDOT, for the 2020-2023 period in the metro freeway network. In this study, the raw work-zone data up to December 2023 are manually processed and organized into an Excel-formatted file, which is then used by the work-zone data script written for populating the T-database. Figure 2.3.3 shows a sample screenshot of the Excel-formatted, work-zone data file processed in this study. The entire set of the Excel-formatted work-zone data file processed in this task is attached in Appendix. The snippets of the script used for populating the T-database containing the work-zone data processed in this task. The updated T-database stores all the work-zone data in the metro freeways from 4/2012 to 12/2023.



Figure 2.3.1 Construction-Project Maps in the Metro Freeway Network (Source: Metro District, MnDOT)

OBJECTID	SP_NBR	CONSTRUCTION_RESIDENT_ENGINEER	PROJROADS	PROJECT_ID	PROJECT_VERSION	PUBLIC_PROJECT_DESCRIPTION	TECHNICAL_PROJECT_DESC	PROGRAM_CODE	PRIM
51	0202-108	Penn, Dan	US10	1033451	10	Reconstruct interchange at Main St, o	**PRS**TED**US 10 169 FROM ANOK/	AM-Agreements Municipal	TRAFFIC CON
100	0215-76	Penn, Dan	US10	1032428	10	Replace 4 bridges and rehabilitate 2 t	**CHAP 3**US10, FROM 0.25 MI EAST	MC-Major Construction	BRPC - BRIDO
309	1901-192	Asche, Greg	MN13	1069183	5	Repair/replace drainage infrastructure	MN13, AT CSAH 26 (LONE OAK RD) IN	DR-Drainage	DRAINAGE, TI
320	1906-74	Asche, Greg	US52	1078443	7	Add crossovers and temporary should	US52, FROM CR86 HAMPTON TO CSAF	RS-Resurfacing	PAVEMENT R
331	1909-99	Asche, Greg	MN55	1038362	12	Resurface, drainage, ADA, rehab brid	MN55 & MN62, FROM E END BRIDGE (	RC-Reconstruction	, BRRH - BRIC
336	1910-56	Asche, Greg	MN55	1070023	4	Repair drainage Hwy 55 at Doyle Pat	**ELLE**MN55, AT DOYLE PATH IN RO	DR-Drainage	DRAINAGE, TI
347	1913-110	Asche, Greg	US61	1095803	2	TH 61 Hastings Bridge #19004 Weari	TH 61 HASTINGS BR #19004 WEARING	BI-Bridge Improvement and Repair	
367	1921-110	Asche, Greg		1070263	3	Construct trail along west side of Hwy	ALONG WEST SIDE MN3 FROM WILLC	LP-Local Partnership Program	BRRH - BRIDO
371	1921-90	Asche, Greg	MN3	1069243	4	Repair drainage, trail (east side), rece	MN3 FROM 0.1 MI N OF BRIDGE 19094	DR-Drainage	DRAINAGE, TI
429	2706-239	Benjamin, Elizabeth	MN7	1032243	12	Bituminous mill and overlay, micro sur	MN 7, FROM 0.07 MI W OF CHRISTMA	RS-Resurfacing	PAVEMENT R
436	2710-47	Barrett, Steven	MN65	1030651	10	Rehabilitate bridge on Hwy 65 at 3rd /	**AC**CHAP 3**CMGC** MN 65, AT BR	BI-Bridge Improvement and Repair	BRRH - BRIDO
465	2724-124	Nelson, Tim	MN55	1032164	9	Redeck of bridges, pavement repair, r	MN55, FROM E OF 13TH AVE TO N OF	BI-Bridge Improvement and Repair	BRRH - BRIDO
466	2724-126	Nelson, Tim	MN55	1032240	6	Resurface, repair sidewalk, pedestrial	MN55 FROM 0.04 MI N OF 32ND ST TO	RS-Resurfacing	PAVEMENT R
509	2750-97	Penn, Dan	US169	1048143	4	Install approximately 5.6 miles of cable	US 169 FROM 85TH ST IN BROOKLYN	SH-Safety Improvements	APPURTENAN
511	2751-58	Nelson, Tim	MN55	1071383	5	Remove remainder of bridge tunnel,	**ELLE**MN55, BASSETT CREEK TUNI	BR-Bridge Replacement	BRPC - BRIDC
519	2758-77	Asche, Greg	MN77	1024759	8	Resurface, extend right turn lane on H	MN 77, FROM N END OF MN RIVER BR	RS-Resurfacing	PAVEMENT R
527	2763-59	Benjamin, Elizabeth	US212	1032429	7	Replace lighting on US Hwy 212 at Sh	**CHAP 3**US 212 AT SHADY OAK RO.	SC-Safety Improvements	APPURTENAN
531	2770-05	Nelson, Tim		1069883	5	Rehab bridge piers on bridges 27816	US952A OVER MULTIPLE ROADWAYS	BI-Bridge Improvement and Repair	BRRH - BRIDO
536	2771-117	Penn, Dan	MN610	1079403	8	Construct traffic management system	MN610, FROM US169 IN BROOKLYN P/	TM-Traffic Management	INTELLIGENT
546	2772-121	Benjamin, Elizabeth	US169	1024765	7	Construct noise wall on NB Hwy 169 f	**CHAP 3**US 169, NB US169 FROM V.	NO-Noise Walls	APPURTENAN
613	2782-327	Barrett, Steven	135W	1024940	8	Construct MnPASS lane; reconstruct	135W, FROM 43RD ST TO 11TH AVE, W	MC-Major Construction	PAVEMENT R
616	2782-347	Barrett, Steven	135W	1026980	15	Construct stormwater holding cavern s	**AC**CMGC**PODI** I35W NB, AT 42I	DR-Drainage	DRAINAGE
637	2783-176	Barrett, Steven	135W	1025025	6	Landscape on 135W under 5th St ped	135W, UNDER PED BRIDGE #27987 AT	RB-Rest Area/Beautification	APPURTENAN
688	6212-194	Embacher, Eric	MN36	1077604	5	Replace signal on Hwy 36 at Hamline	MN36, AT HAMLINE AVE N IN ROSEVIL	SC-Safety Improvements	TRAFFIC CON
701	6216-141	Embacher, Eric	MN51	1049240	8	Install approximately 2.7 miles of cable	MN 51 FROM CR C IN ROSEVILLE TO I	SH-Safety Improvements	APPURTENAN
710	6217-52	Asche, Greg		1069263	5	Repair 2 bridges on Hwy 3 at George	MN 3, AT GEORGE ST AND AT THE UN	BI-Bridge Improvement and Repair	BRRH - BRIDO
779	6282-242	Embacher, Eric	194	1068803	4	Landscape I94 east bound from Prior	194 EB, FROM PRIOR AVE TO FAIRVIE	RB-Rest Area/Beautification	APPURTENAN
797	6283-247	Embacher, Eric	135E, 194	1032009	10	Repair road and bridges on I-94 from	**ELLE**194, FROM 0.2 MI W OF WES'	RC-Reconstruction	BRRH - BRIDO
799	6283-255	Embacher, Eric	135E, 194	1068883	10	Resurface frontage roads and ramps,	194, FROM WESTERN TO US 52 AND O	RS-Resurfacing	PAVEMENT R
817	R995 180	Noleon Tim	1804	1022008	44	Donland cional on LR04 at Citor Lake	IRDA AND SILVED I AVE DO N AND S D	90 Cafely Improvements	TRAFFIC CON

Figure 2.3.2 A Sample Screenshot of the Detailed Information for the Metro Construction Projects

(Source: Metro District, MnDOT)

Rote ID #	Memo	Start	End	Start Station	End Station
SP# 0215-76	No Lane Config Available	03/07/2022	11/01/2023	S940	S943
SP# 1906-74	No Lane Config Available	05/01/2022	11/01/2022	S5211	\$5201
SP# 1909-99	No Lane Config Available	04/01/2022	10/31/2022	S5215	S1166
SP# 1913-110	No Lane Config Available	06/01/2022	10/01/2022	S1594	S2015
SP# 2750-97	No Lane Config Available	09/12/2022	10/15/2022	S1802	S2043
SP# 2758-77	No Lane Config Available	06/01/2023	11/30/2023	S522	\$531
SP# 2763-59	No Lane Config Available	04/12/2022	08/12/2022	S361	\$359
SP# 2771-117	No Lane Config Available	05/01/2022	07/15/2022	S2045	S1006
SP# 2772-121	No Lane Config Available	05/16/2022	11/04/2022	S427	S430
SP# 2782-347	No Lane Config Available	12/01/2019	08/01/2023	S77	\$57
SP# 2783-176	No Lane Config Available	09/15/2020	07/01/2023	S583	\$582
SP# 6212-194	No Lane Config Available	09/01/2022	11/01/2022	\$591	\$593
SP# 6217-52	No Lane Config Available	05/15/2022	08/15/2022	S791	S1069
SP# 6282-242	No Lane Config Available	04/01/2022	07/01/2022	S1809	S1808
SP# 6283-247	No Lane Config Available	07/01/2021	11/01/2022	S644	S642
SP# 6283-255	No Lane Config Available	04/18/2022	07/30/2022	S789	S97
SP# 6285-160	No Lane Config Available	04/01/2022	06/30/2022	S172	S151
SP# 8204-77	No Lane Config Available	04/25/2022	09/16/2022	S1829	S1852
SP# 8282-145	No Lane Config Available	08/01/2022	11/01/2022	S1064	S1359
SP# 8825-879	No Lane Config Available	05/01/2021	06/30/2022	S1457	S1423
SP# 0215-76	No Lane Config Available	03/07/2022	06/15/2024	S940	S942
SP# 157-108-035	No Lane Config Available	08/01/2021	06/30/2023	S922	S921
SP# 1906-71	No Lane Config Available	05/01/2023	11/01/2024	S5201	S5211
SP# 1928-90	No Lane Config Available	09/01/2023	11/01/2023	S1171	S1173
SP# 2735-202	No Lane Config Available	04/01/2023	05/31/2023	S393	S1614
SP# 2748-68	No Lane Config Available	07/01/2023	11/01/2023	S238	S2049
SP# 2772-115	No Lane Config Available	04/01/2023	11/01/2023	S767	S762

Figure 2.3.3 An Example Screenshot of the Excel-formatted Work-Zone Data File



Figure 2.3.4 The Snippets of the Script for Processing Work-Zone Data

wz_group_id	memo text	start_time timestamp without time zone	end_time timestamp without time zone	route1 text	route2 text	reg_date timestamp without time zone	workzone_length double precision
326	No Lan	2021-07-01 00:00:00	2023-04-23 23:55:00	{"class	{"class	2022-11-27 17:53:36.159268	2.01862171622635
325	No Lan	2022-07-01 00:00:00	2023-07-30 23:55:00	{"class	{"class	2022-11-27 17:53:34.102632	2.7869492813839
324	No Lan	2021-08-15 00:00:00	2023-11-01 23:55:00	{"class	{"class	2022-11-27 17:53:32.030979	1.34906695257318
323	No Lan	2023-07-01 00:00:00	2024-06-30 23:55:00	{"class	{"class	2022-11-27 17:53:29.974882	5.85123258556678
322	No Lan	2023-05-15 00:00:00	2023-09-30 23:55:00	{"class	{"class	2022-11-27 17:53:27.85414	1.67575790818557
321	No Lan	2023-07-01 00:00:00	2023-11-15 23:55:00	{"class	{"class	2022-11-27 17:53:25.59036	0.949495823818629
320	No Lan	2023-08-15 00:00:00	2023-11-01 23:55:00	{"class	{"class	2022-11-27 17:53:23.409651	0.887851714596173
319	No Lan	2023-04-01 00:00:00	2024-11-01 23:55:00	{"class	{"class	2022-11-27 17:53:21.301621	9.04663658694889
318	No Lan	2023-07-01 00:00:00	2024-06-30 23:55:00	{"class	{"class	2022-11-27 17:53:19.192197	4.02188548798134
317	No Lan	2023-08-15 00:00:00	2024-06-15 23:55:00	{"class	{"class	2022-11-27 17:53:17.109924	10.367899915307
316	No Lan	2023-07-01 00:00:00	2024-06-30 23:55:00	{"class	{"class	2022-11-27 17:53:14.974924	4.75948225806303
315	No Lan	2023-09-01 00:00:00	2023-11-01 23:55:00	{"class	{"class	2022-11-27 17:53:12.888866	0.763582436748481
314	No Lan	2022-04-15 00:00:00	2023-11-01 23:55:00	{"class	{"class	2022-11-27 17:53:10.830009	1.40698564906313
313	No Lan	2023-04-15 00:00:00	2023-07-15 23:55:00	{"class	{"class	2022-11-27 17:53:08.76575	0.840511163152236
312	No Lan	2023-04-01 00:00:00	2024-11-01 23:55:00	{"class	{"class	2022-11-27 17:53:06.692411	3.11454715143925
311	No Lan	2023-11-15 00:00:00	2026-11-15 23:55:00	{"class	{"class	2022-11-27 17:53:04.624239	5.6730279726431

Figure 2.3.5 A Screenshot of the T-database Showing a Portion of the Work-Zone Data

### 2.4 Collection and Processing of Winter Road-Condition Data

Figure 2.4.1 shows a portion of the metro-area snow-event data, for the period of April 2019 – April 2022, provided by the MnDOT Metro District, which has also provided with the location of each snow-plow route in terms of cross–street information.

First, for each snow-plow route, the detector-station IDs at both starting/ending points were determined by examining the cross-street information and the detector-station map in TICAS, Traffic Information-Condition Analysis System [3]. Figure 2.4.2 includes a portion of the metro-freeway snow-plow route data file with the detector-station IDs at start/end locations. Next, the specific snow-event data, provided by the MnDOT Metro District, for each of the snow-plow routes on the metro freeways were converted into the Excel-formatted, snow-event data file, whose pre-defined data structure includes date/time, route ID and bare-lane lost/regain times for each event. Figure 2.4.3 includes a portion of the Excel-formatted, 2018-2019 snow-event data file for the metro-freeway snow-plow routes processed in this study. The Excel-formatted data file was then used by the snow-event data script, shown in Figure 2.4.4, to populate the T-database. Figure 2.4.5 shows a screenshot of the T-database populated with the snow-event data for the snow-plow routes on the metro road-condition data for the snow-plow routes on the metro-freeways from 10/2012 until 12/2023.

District Na 🗸	aintenance Area N <del>-</del>	ruck Station Na <mark>-</mark>	Project 🗸	Project Maintenance Route	■ervice Level Na	ent Begin Date 🔽	rent End Date T🔽	ent Duration H💌
Metro District	Maintenance Area Metro	Mendota	TP9M4941	MN RIVER - 34TH AVE	SUPER COMMUTER	11/13/21 4:00 PM	11/13/21 6:00 PM	2
Metro District	Maintenance Area Metro	Mendota	TP9M4942	TH494 000+00.000 001+00.622	SUPER COMMUTER	11/13/21 4:00 PM	11/13/21 6:00 PM	2
Metro District	Maintenance Area Metro	Shakopee	TP5P0251	JCT OF US169-S JCT MN20	RURAL COMMUTER	11/13/21 2:30 PM	11/13/21 10:30 PM	8
Metro District	Maintenance Area Metro	Chaska Truck Station	TP5M0071	JCT MN 25-JCT MN41 IN SHOREWOOD	URBAN COMMUTER	11/13/21 2:00 PM	11/13/21 11:00 PM	9
Metro District	Maintenance Area Metro	Chaska Truck Station	TP5M0071	JCT MN 25-JCT MN41 IN SHOREWOOD	URBAN COMMUTER	11/13/21 2:00 PM	11/13/21 11:00 PM	9
Metro District	Maintenance Area Metro	Chaska Truck Station	TP5M0411	JCT US 169-JCT MN22	URBAN COMMUTER	11/13/21 2:00 PM	11/13/21 11:00 PM	9
Metro District	Maintenance Area Metro	Chaska Truck Station	TP5M0411	JCT US 169-JCT MN23	URBAN COMMUTER	11/13/21 2:00 PM	11/13/21 11:00 PM	9
Metro District	Maintenance Area Metro	Chaska Truck Station	TP5M1691	JCT MN 41-PIONEER TRAIL	SUPER COMMUTER	11/13/21 2:00 PM	11/13/21 11:00 PM	9
Metro District	Maintenance Area Metro	Chaska Truck Station	TP5M1691	JCT MN 41-PIONEER TRAIL	SUPER COMMUTER	11/13/21 2:00 PM	11/13/21 11:00 PM	9
Metro District	Maintenance Area Metro	Chaska Truck Station	TP5M2121	W JCT MN 5/25-LYMAN BLVD OVERPASS	URBAN COMMUTER	11/13/21 2:00 PM	11/13/21 11:00 PM	9
Metro District	Maintenance Area Metro	Chaska Truck Station	TP5M2121	W JCT MN 5/25-LYMAN BLVD OVERPASS	URBAN COMMUTER	11/13/21 2:00 PM	11/13/21 11:00 PM	9
Metro District	Maintenance Area Metro	Mendota	TP9M0031	DAKOTA CTY RD 42-JCT MN125	URBAN COMMUTER	11/13/21 3:00 PM	11/13/21 11:00 PM	8
Metro District	Maintenance Area Metro	Mendota	TP9M0131	JCT MN77-ANNAPOLIS ST	URBAN COMMUTER	11/13/21 4:00 PM	11/13/21 11:00 PM	7
Metro District	Maintenance Area Metro	Mendota	TP9M1491	JCT MN3-S JCT MN55	URBAN COMMUTER	11/13/21 3:00 PM	11/13/21 11:00 PM	8
Metro District	Maintenance Area Metro	Mendota	TP9M1491	JCT MN3-S JCT MN55	URBAN COMMUTER	11/13/21 3:00 PM	11/13/21 11:00 PM	8
Metro District	Maintenance Area Metro	Plymouth	TP5C0551	MOHAWK DR-194	SUPER COMMUTER		11/13/21 11:00 PM	0
Metro District	Maintenance Area Metro	Plymouth	TP5C3941	JCT 1494-JCT MN110	SUPER COMMUTER		11/13/21 11:00 PM	0
Metro District	Maintenance Area Metro	Plymouth	TP5C4941	JCT MN 7-BASS LAKE ROAD	SUPER COMMUTER		11/13/21 11:00 PM	0
Metro District	Maintenance Area Metro	Shakopee	TP5P0131	E JCT MN19-JCT MN101 IN SAVAGE	URBAN COMMUTER	11/13/21 4:10 PM	11/13/21 11:00 PM	6.83
Metro District	Maintenance Area Metro	Shakopee	TP5P1691	MN282/2ND ST-JCT MN 41/CSAH93	SUPER COMMUTER	11/13/21 3:30 PM	11/13/21 11:00 PM	7.5
Metro District	Maintenance Area Metro	Shakopee	TP5P2821	JCT US169 IN JORDAN-JCT MN28	RURAL COMMUTER	11/13/21 3:30 PM	11/13/21 11:15 PM	7.75

Figure 2.4.1 A Portion of the Snow-Event Data for the Metro Freeways (Source: Metro District, MnDOT)

Route	From	To	MP Begin 👻	MP End 🔻		Start Statio	End Station -	Subarea 👻
TP5A0101	WB exit ramp to US 169 NB/MN 101 SB	Creek, 1/2 way between Main St and	214.553	228.156	US10 EB	S1214	S951	ANOKA
TP9D0101	Junction I35W NB/US910A	US10 becomes CO with I694	238.393	240+01.183	US10 EB	S2039	S1825	ARDEN HILLS
TP9D35W1	South end of Bridge 27897 over Industrial	N jct I-35, N jct I-35E	21.507	41.743	135W NB	S573	S1561	ARDEN HILLS
TP9D6941	BR# 9389 under 5th Ave NW (Old Hwy 8)	W end BR # 9209/9210 over Island	40.426	43.45	1694 EB	S177	S1079	ARDEN HILLS
TP5H0941	Junction MNTH - 252	Hennepin/Ramsey County Line	224.977	236.319	194 EB	S240	S775	CAMDEN
TP5H35W1	Chicago Avenue in Minneapolis	South end of Bridge 27897 over	17.129	21.507	35W NB	S565	S573	CAMDEN
TP5H3941	Junction T.H. 100 in Golden Valley	Washington Avenue North in	5.925	9.735	1394 EB	S281	S291	CAMDEN
TP5J0621	West Junction USTH 212/Bri#27080	Junction of MN 55	105.867	115.982	62 EB	S311	S1135	CEDAR
TP5J0771	West 138th Street in Apple Valley	BR# 27021 under MN 62 WB	0.898	11.403	77 NB	S920	S531	CEDAR
TP5J35W1	Junction I-494 in Bloomington	Chicago Avenue in Minneapolis	8.741	17.129	35W NB	S45	S565	CEDAR
TP5J4941	East end of Bridge 27765 over 34th Avenue	West Junction MNTH 5 in Eden Prairie	1.608	11.921	494 EB	S474	S494	CEDAR
TP5N0621	Junction I-494 in Eden Prairie	W. junction US212/EB under US169 NB	103.592	105.867	62 EB	S301	S311	EDEN PRAIRIE
TP5N1692	Pioneer Trail CSAH 1	Junction T.H. 55 in Plymouth	118.682	130.940	169 NB	S1143	S442	EDEN PRAIRIE
TP5N2121	Lyman Blvd overpass	East junction MN 62	152.809	162.482	TH 212 EB	S1379	S312	EDEN PRAIRIE
TP5N4941	West Junction US 212 in Eden Prairie	Junction T.H. 7 westbound - Bridge	11.921	16.259	494 NB	S484	S512	EDEN PRAIRIE
TP9B0351	North Junction I-35E and I-35W	Washington-Chisago County Line	127.420	132.939	135 NB	S1591	S1511	FOREST LAKE
TP9B35E1	North end of bridge 9568 northbound over	North Junction I-35 and I-35W	115.742	127.420	35E NB	S1485	S1503	FOREST LAKE
TP5E0941	BR #27707 under Boone Ave	Junction MNTH - 252	219.520	224.977	94 EB	S223	S240	GOLDEN
TP5E1001	Junction I494/Bridge #27V38	Junction I-694 in Brooklyn Center	0.000	16.158	TH 100 NB	S375	S1614	GOLDEN
TP5E6941	Junction I-94 in Brooklyn Center	BR # 9389 under 5th Ave NW (Old Hwy	34.197	40.426	694 EB	S2059	S179	GOLDEN
TP9P35E1	South Junction I-35 / I-35W	Bridge#19816 under Cliff Road	88.267	93.744	35 E NB	S870	S879	LAKEVILLE
TP9P35W1	South Junction I-35 / I-35E, end Bridge #	Junction I-494 in Bloomington/Bridge	0.000	8.741	35W NB	S911	S43	LAKEVILLE
TP9P35SI	SB exit ramp to MN 19	South Jct I-35E / I-35W	69.774	88.267	135 NB	S1585	S1096	LAKEVILLE
TP5B0941	Wright/Hennepin County line, NW end of	BR #27707 under Boone Ave	205.367	219.520	194 EB	S1734	S223	MAPLE GROVE
TP5B1011	Junction Hennepin Co. Rd. 81 in Rogers Jct.	Wright / Sherburne County Line (North	39.640	46.350	TH 101 NB	S1430	S1435	MAPLE GROVE
TP5B1691	Junction T.H. 55 in Plymouth	SB exit ramp to WB MN610	130.940	141.627	US 169 NB	S442	S1799	MAPLE GROVE

Figure 2.4.2 A portion of the Metro-Freeway Snow-Plow Route Location Data with Station IDs

(Source: Metro District, MnDOT. Start/End Point Detector-Station IDs added in this task)

Event Start	Event End	Affected Routes	Lane Lost Time	Lane Regain Time
11/28/2018 2:00:00 PM	11/29/2018 3:45:00 AM	TP5A0101	11/29/2018 1:00:00 AM	11/29/2018 5:00:00 AM
12/1/2018 4:00:00 PM	12/2/2018 11:00:00 AM	TP5A0101	12/1/2018 5:00:00 PM	12/1/2018 6:00:00 PM
12/26/2018 8:30:00 PM	12/27/2018 6:00:00 AM	TP5A0101	12/26/2018 9:30:00 PM	12/27/2018 4:30:00 AM
12/27/2018 8:45:00 PM	12/28/2018 6:00:00 AM	TP5A0101	12/27/2018 9:30:00 PM	12/28/2018 4:30:00 AM
1/27/2019 6:30:00 PM	1/28/2019 8:00:00 AM	TP5A0101	1/27/2019 8:00:00 PM	1/28/2019 10:00:00 AM
2/5/2019 8:30:00 AM	2/5/2019 8:00:00 PM	TP5A0101	2/5/2019 9:30:00 AM	2/5/2019 3:00:00 PM
2/6/2019 8:00:00 PM	2/7/2019 8:30:00 PM	TP5A0101	2/6/2019 8:20:00 PM	2/7/2019 3:00:00 AM
2/10/2019 7:00:00 AM	2/10/2019 5:30:00 PM	TP5A0101	2/10/2019 9:30:00 AM	2/10/2019 7:30:00 PM
2/11/2019 11:30:00 PM	2/12/2019 4:30:00 PM	TP5A0101	2/12/2019 1:00:00 AM	2/12/2019 5:30:00 PM
2/20/2019 4:00:00 AM	2/21/2019 12:00:00 AM	TP5A0101	2/20/2019 4:40:00 AM	2/21/2019 1:00:00 AM
2/23/2019 2:00:00 AM	2/23/2019 7:00:00 AM	TP5A0101	2/23/2019 2:30:00 AM	2/23/2019 8:00:00 AM
2/23/2019 8:30:00 PM	2/24/2019 4:00:00 AM	TP5A0101	2/23/2019 10:30:00 PM	2/24/2019 11:00:00 PM
2/26/2019 11:30:00 PM	2/27/2019 5:30:00 AM	TP5A0101	2/26/2019 12:30:00 PM	2/27/2019 7:30:00 AM
3/1/2019 10:30:00 AM	3/1/2019 4:00:00 PM	TP5A0101	3/1/2019 11:15:00 AM	3/2/2019 8:00:00 AM
12/27/2018 10:30:00 PM	12/28/2018 2:00:00 AM	TP5H0941	12/28/2018 1:00:00 AM	12/28/2018 4:30:00 AM
12/26/2018 7:00:00 PM	12/27/2018 1:00:00 AM	TP5H0941	12/26/2018 8:00:00 PM	12/27/2018 4:00:00 AM
12/22/2018 10:00:00 PM	12/23/2018 1:30:00 AM	TP5H0941	12/22/2018 11:00:00 PM	12/23/2018 3:00:00 AM
3/1/2019 9:00:00 AM	3/1/2019 9:00:00 PM	TP5H0941	3/1/2019 10:00:00 AM	3/2/2019 3:00:00 AM
2/23/2019 1:30:00 AM	2/24/2019 3:00:00 AM	TP5H0941	2/23/2019 2:00:00 AM	2/24/2019 7:00:00 AM
2/20/2019 3:00:00 AM	2/20/2019 11:30:00 PM	TP5H0941	2/20/2019 3:30:00 AM	2/20/2019 9:00:00 PM
12/27/2018 10:30:00 PM	12/28/2018 2:00:00 AM	TP5H35W1	12/28/2018 1:00:00 AM	12/28/2018 4:30:00 AM
12/26/2018 7:00:00 PM	12/27/2018 1:00:00 AM	TP5H35W1	12/26/2018 8:00:00 PM	12/27/2018 4:00:00 AM
12/22/2018 10:00:00 PM	12/23/2018 1:30:00 AM	TP5H35W1	12/22/2018 11:00:00 PM	12/23/2018 3:00:00 AM
3/1/2019 9:00:00 AM	3/1/2019 9:00:00 PM	TP5H35W1	3/1/2019 10:00:00 AM	3/2/2019 3:00:00 AM

Figure 2.4.3 A Sample Snow-Event Data for 2018-19 in Excel Format Processed for T-database

```
populate_snow_event_data - Notepad
File Edit Format View Help
import sys
sys.path.append("Server/src")
import global settings
from pyticas import ticas
from pyticas.infra import Infra
from pyticas_tetres import api_urls_admin
from tetres_data_populator.snow_event_data.snow_event_api_reader import SnowEventAPIReader
from tetres_data_populator.snow_event_data.snow_event_api_writer import SnowEventAPIWriter
from tetres_data_populator.snow_event_data.snow_event_excel_reader import SnowEventExcelReader
    _name_
                  _main_
if
  _
    __name__ == ___main___:
ticas.initialize(global_settings.DATA_PATH)
    print(global_settings.DATA_PATH)
    infra = Infra.get_infra()
    base_url = "http://localhost:5000"
    only_file_name = input("Enter the name of the snow event excel file: ")
snow_event_filename = "Server/src/tetres_data_populator/excel_data/snow_event_data/{}".format(only_file_name)
snow_event_excel_reader = SnowEventExcelReader(filename=snow_event_filename)
    # adding snow route data
    snow_event_api_writer = SnowEventAPIWriter(infra)
    api_reader_class=SnowEventAPIReader)
    # adding snow event data
    # adding snow management data
    data_type="snow_management",
api_reader_class=SnowEventAPIReader)
```

Figure 2.4.4 The Snippets of the Script for Populating T-database with Snow-Event Data

id [PK] integer	start_time timestamp without time zone	end_time timestamp without time zone	reg_date timestamp without time zone
4/68	2021-12-28 10:15:00	2021-12-28 15:30:00	2022-11-28 20:31:30.587804
4767	2021-12-28 09:30:00	2021-12-28 15:00:00	2022-11-28 20:31:28.517967
4766	2021-12-26 14:30:00	2021-12-27 21:30:00	2022-11-28 20:31:26.463734
4765	2021-12-26 18:45:00	2021-12-27 09:00:00	2022-11-28 20:31:24.40492
4764	2021-12-26 19:00:00	2021-12-27 08:15:00	2022-11-28 20:31:22.351324
4763	2021-12-26 18:45:00	2021-12-27 08:00:00	2022-11-28 20:31:20.29718
4762	2021-12-26 18:45:00	2021-12-27 07:30:00	2022-11-28 20:31:18.243063
4761	2021-12-26 18:30:00	2021-12-27 07:30:00	2022-11-28 20:31:16.200203
4760	2021-12-26 19:00:00	2021-12-27 07:00:00	2022-11-28 20:31:13.480339
4759	2021-12-26 19:15:00	2021-12-27 07:00:00	2022-11-28 20:31:11.430401
4758	2021-12-26 19:00:00	2021-12-27 06:30:00	2022-11-28 20:31:09.367235
4757	2021-12-26 18:05:00	2021-12-27 06:30:00	2022-11-28 20:31:06.692537
4756	2021-12-26 17:30:00	2021-12-27 06:00:00	2022-11-28 20:31:04.630459
4755	2021-12-26 17:15:00	2021-12-27 06:00:00	2022-11-28 20:31:02.570199
4754	2021-12-26 18:45:00	2021-12-27 06:00:00	2022-11-28 20:31:00.408383
4753	2021-12-26 17:50:00	2021-12-27 06:00:00	2022-11-28 20:30:58.363742
4752	2021-12-26 18:05:00	2021-12-27 06:00:00	2022-11-28 20:30:55.950984

Figure 2.4.5 Screenshot of T-database with a Portion of Populated Snow-Event Data

## 2.5 Collection and Processing of Incident Data

Figure 2.5.1 shows the structure of the incident data processing module developed in the previous phase of this study. As noted in the figure, the current process for populating the T-database with the metro-freeway requires the incident data from two external sources, i.e., IRIS (Intelligent Road Information System) from RTMC, MnDOT, and CAD (Computer-Aided Dispatch) system from the Department of Public Safety. After both IRIS and CAD incident-database files are imported and integrated in a combined data format, the Incident-Impact data, i.e., lane-closure status, and the Lane-type information are extracted from the IRIS-Incident database and merged onto the data from the CAD system, which is considered as the main data source for incidents. The integrated-incident data is then stored in the T-database. Figure 2.5.2 includes the snippets of the incident-data script, designed to read the imported database files, and to populate the T-database with the extracted data from both databases.



Figure 2.5.1 Structure of the Incident Data Reader/T-Database Population Module



#### Figure 2.5.2 Snippets of the Script for Populating T-database with Incident data

In this study, after examining both IRIS and CAD-incident database files, it was noticed that the formats of both database files were changed from those used in the previous phase of this study. As shown in Figure 2.5.3, the CAD database used in the previous phase had 11 columns, while the CAD database provided by RTMC, MnDOT, for the current study, has 16 columns. Further, the recent database from IRIS, i.e., tms database, was noted to have one more schema, named as 'cap', than the 2020-version of the tms database. Therefore, in this study, those additional columns and schema were deleted from the recent IRIS and CAD databases, so that the current script can be applicable for populating T-database with the new data sets. Figure 2.5.4 shows a portion of the T-database populated with the new set of the incident

data. The updated T-database contains the incident data for the metro freeways from 1/2010 until December 2023.



Figure 2.5.3 Format Changes in CAD and IRIS Incident Databases

cdts timestamp without time zone	udts timestamp without time zone	xdts timestamp without time zone	lat double precision	lon double precision	xstreet1 character varying (40)	xstreet2 character varying (40)
2022-09-30 20:15:44	2022-09-30 20:15:58	2022-09-30 20:42:52	44.96675	-93.08932	SB 35E I	CAYUGA ST E
2022-09-30 20:07:50	2022-09-30 20:08:16	2022-10-03 12:50:32	44.40554	-95.12763	71 HWY	68 HWY
2022-09-30 20:06:36	2022-09-30 20:06:57	2022-09-30 20:37:45	44.96569	-93.26254	NB 35W I	CHICAGO AVE S
2022-09-30 20:05:14	2022-09-30 20:06:32	2022-09-30 20:37:44	45.05031	-93.03594	NB 61 HWY	244 HWY
2022-09-30 20:03:31	2022-09-30 20:03:48	2022-10-03 12:52:49	45.13992	-95.00592	SB 71 HWY TO WB 12 HWY	CIVIC CENTER DR
2022-09-30 20:02:32	2022-09-30 20:03:01	2022-10-03 12:52:50	45.45423	-94.0067	70TH AVE SE	75TH AVE
2022-09-30 20:01:47	2022-09-30 20:02:03	2022-10-03 12:52:50	46.57558	-95.54408	WB 10 HWY TO 78 HWY RMP	80 CR
2022-09-30 20:01:33	2022-09-30 20:02:20	2022-09-30 21:09:20	44.68131	-93.29359	SB 35 I	185TH ST W
2022-09-30 20:00:57	2022-09-30 20:13:34	2022-10-03 12:52:50	44.18588	-91.85198	CHANNEL VIEW RD	BENNETT AVE
2022-09-30 20:00:15	2022-09-30 20:16:23	2022-10-03 12:52:50	45.2815	-93.549	WB 10 HWY NW	171ST AVE NW
2022-09-30 19:58:10	2022-09-30 20:36:42	2022-10-03 12:52:51	47.00427	-93.26094	65 HWY	200 HWY
2022-09-30 19:56:25	2022-09-30 20:04:39	2022-10-03 12:52:51	46.93471	-95.91028	59 HWY	12 CR
2022-09-30 19:44:17	2022-09-30 19:45:42	2022-10-03 12:52:51	46.35774	-93.74939	37 CR	PINE LAKE RD
2022-09-30 19:42:02	2022-09-30 19:42:39	2022-09-30 20:32:28	45.65086	-93.19324	65 HWY	FJELDHEIM RD NE
2022-09-30 19:35:23	2022-09-30 19:37:40	2022-09-30 23:41:35	45.07616	-93.35565	EB 694 I	ZANE AVE N

Figure 2.5.4 A Screenshot of T-database Showing a Portion of Populated Incident Data

#### 2.6 Collection and Processing of Weather Data

As noted previously, in TeTRES, the weather data is directly downloaded from the data archive of the NOAA, *National Oceanic and Atmospheric Administration*, to the T-database. Figure 2.6.1 shows the script used for downloading the weather data for the period of 11/2020 – 12/2023. from the weather-

data stations in the metro-freeway network and for populating the T-database. A screenshot of the sample weather data downloaded into the T-database is shown in Figure 2.6.2.



#### Figure 2.6.1 The Snippets of Script for Downloading Weather Data and Populating T-Database

dtime timestamp without time zone 🗸	precip double precision	, precip_type character varying (4)	precip_intensity character varying (4)	precip_qc character varying (4)	double precision	visibility_qc character varying (4)	, obscuration character varying (4)	descriptor character varying (4)	air_temp double precision	air_temp_qc character varying (4)
2022-08-29 21:55:00		99	9	3	9.999723503	5	9	9	64.4	C
2022-08-29 21:15:00		99	9	3	9.999723503	5	9	9	64.4	5
2022-08-29 20:55:00		99	9	3	9.999723503	5	9	9	66.2	C
2022-08-29 19:55:00		99	9	3	9.999723503	5	9	9	66.2	C
2022-08-29 19:35:00		99	9	3	9.999723503	5	9	9	68	5
2022-08-29 19:15:00		99	9	3	9.999723503	5	9	9	69.8	5
2022-08-29 18:55:00		99	9	3	9.999723503	5	9	9	69.8	C
2022-08-29 18:15:00		99	9	3	9.999723503	5	9	9	71.6	5
2022-08-29 17:35:00		99	9	3	9.999723503	5	9	9	73.4	5
2022-08-29 17:15:00		99	9	3	9.999723503	5	9	9	75.2	5
2022-08-29 16:55:00		99	9	3	9.999723503	5	9	9	75.2	C
2022-08-29 16:35:00		99	9	3	9.999723503	5	9	9	75.2	5
2022-08-29 15:35:00		99	9	3	9.999723503	1	9	9	77	1
2022-08-29 15:15:00		99	9	3	9.999723503	1	9	9	75.2	1
2022-08-29 14:55:00	(null)	99	9	3	9.999723503	1	9	9	75.2	1
# 2.7 Summary

As described in the previous sections, in this study, the TeTRES database was populated with a new set of data to continuously cover the period from the end of the previous phase until December 2023. The resulting, updated TeTRES database contains the following data sets:

- Special-event data: Sport-Events for 1/2012-12/2023, Non-Sport Event for 2016 2023
- Work-zone data from 4/2012 12/2023
- Winter-road condition data for Metro Freeway-Plow Routes from 10/2012 12/2023
- Weather data from NOAA for 1/2010 912/2023
- Incident data from CAD/IRIS database for 1/2010 12/2023
- Traffic-detector data (stored separately in a local hard disk): 1/2010 12/2023.

The TeTRES database updated with the above datasets is applied for estimating the travel-time reliability and traffic-flow measures at the metro freeway corridors in the next chapter.

# Chapter 3: ESTIMATION AND ANALYSIS OF TRAVEL-TIME TTR MEASURES AND TRENDS FOR THE METRO FREEWAY CORRIDORS AND NETWORK

## **3.1 Introduction**

In this chapter, the updated TeTRES database, populated with the new sets of the traffic and non-traffic data up to December 2023, is applied to estimate the travel-time reliability (TTR) and traffic-flow measures of the freeway corridors in the metro network. Figure 3.1.1 shows a total of 37 corridors with 76 directional routes, whose monthly and yearly TTR and traffic-flow measures of effectiveness (MoE) are estimated during morning or afternoon peak periods from January 2018 to December 2023. It needs to be noted that each corridor has two directional routes for morning and afternoon peak periods, while the I-94 Corridor between St. Paul and Minneapolis downtowns have a total of 4 directional routes, i.e., each directional route have been determined in cooperation with the technical advisory panel of this study. Table 3.1.1 includes the detector-station IDs at the boundaries of each directional route.



Figure 3.1.1 Freeway Corridors defined for TTR and Traffic-Flow MoE Estimation

Corridor	Description	Start	End	Length (Miles)
I-394 (EB)	I494 to TH169	S269	S274	2.7
I-394 (EB)	TH169 to MPLS DT	S274	S290	5.5
I-494 (SB)	1694 to TH212	S209	S473	15.5
I-494 (WB)	194 to 135E	S1029	S864	12.4
I-494 (WB)	I35E to I35W	S864	S119	7.8
I-494 (WB)	I35W to TH212	S119	S483	5.9
I-694 (NB)	194 to TH36	S1027	S1418	4.9
I-694 (WB)	TH36 to I35E	S1419	S1459	6.5
I-694 (WB)	135E to TH10	S1461	S1089	3.8
I-694 (WB)	TH10 to TH252	S1089	S144	5.5
I-94 (EB)	Rogers to I494	S1115	S211	9.1

## Table 3.1.1 Start/End Stations of Each Corridor Directional Route

I-94 (EB)	MPLS DT to SP DT	S110	S499	8.4
I-94 (EB)	MPLS DT to SP DT	S110	S499	8.4
I-94 (EB)	SP DT to I694	S499	S2151	7.3
I-94 (EB)	1694 to TH95	S2151	S1358	9.2
I-94 (EB)	I494 to TH252	S211	S170	8
I-94 (EB)	TH252 to MPLS DT	S170	S110	8.1
I-94 (WB)	1494 to Rogers	S208	S1112	8.4
I-94 (WB)	SP DT to MPLS DT	S500	S1943	8.5
I-94 (WB)	SP DT to MPLS DT	S500	S1943	8.5
I-94 (WB)	TH95 to 1694	S1359	S1062	9
I-94 (WB)	I694 to SP DT	S1062	S500	7.5
I-94 (WB)	MPLS DT to TH252	S76	S159	8
I-94 (WB)	TH252 to 1494	S159	S216	7.1
T.H.10 (EB)	TH169 to TH610	S940	S954	7.5
T.H.10 (EB)	TH610 to I694	S954	S1825	7.3
T.H.100 (SB)	1694 to 1394	S1615	S405	6.8
T.H.100 (SB)	1394 to 1494	S405	S421	7.7
T.H.36 (WB)	I35E to I35W	S609	S618	4.4
T.H.36 (WB)	1694 to 135E	S1425	S608	6.4
T.H.52 (NB)	TH55 to SP DT	S1166	S1178	8.2
Т.Н.610 (ЕВ)	194 to TH10	S1954	S966	11.7
T.H.62 (WB)	135W to 1494	S127	S369	6.7
T.H.62 (WB)	TH55 to I35W	S330	S127	4.2
T.H.77 (NB)	127th to TH62	S799	S531	6.1
U.S.169 (SB)	TH610 to I394	S1795	S448	10.9
U.S.169 (SB)	I394 to TH13	S448	S1626	16.2

For each directional route of the individual corridors shown in Figure 3.1.1, the TTR measures, based on the travel times calculated every 5-minute interval for each route, were estimated using TeTRES under different operating conditions, and their monthly and yearly trends were analyzed. It needs to be noted that, in this analysis, only regular non-holiday weekdays, i.e., Tuesdays, Wednesdays, Thursdays, were included for estimating TTR measures. In addition, a set of the commonly used traffic-flow MoEs were also estimated and presented for each route. The specific measures and operating conditions used in this analysis are as follows:

#### Travel-time TTR Measures:

Buffer Index (BI, 95<sup>th</sup> %ile) = (95<sup>th</sup> %ile Travel Time – Average Travel Time)/(Average Travel Time) Planning Index (PI, 95<sup>th</sup> %ile) = 95<sup>th</sup> %ile Travel Time / Free-Flow Travel Time *Travel Rate (TR, 95<sup>th</sup> %ile, minutes/mile) = 95<sup>th</sup> %ile Travel Time / Route Length* 

#### **Operating Conditions**

Weather: All, Dry, Rain, Snow

Incident: All, No-Incident (N), Property Damage Only (PD), Severe/Fatal (INJ, FA)

Work Zone: All, No-Work Zone (N), Light Impact-WZ (L), Medium-Heavy Effect WZ (M, H)

Special Event: No-Event, Small (< 20,000 attendees), Medium-Large (≥ 20,000 attendees) events

**Peak Periods**: Morning: 6:00 – 9:00 a.m. Afternoon: 3:30 – 6:30 p.m.

Traffic-Flow Measures of Effectiveness: VMT (Vehicle-Miles Traveled), VHT (Vehicle-Hours Traveled),

#### DVH (Delayed Vehicle-Hours)

Further, the monthly and yearly values of the network-wide, weighted-average TTR measures, using the VMT of each route as the weight, under 'All' conditions were calculated to analyze the network-wide TTR trends for both morning and evening routes. For example, the 'network-wide average buffer index' is calculated as follows:

Network-wide Average Buffer Index (BI) =  $\frac{R1 VMT * R1 BI + R2 VMT * R2 BI + ---- + RnVMT * Rn BI}{R1 VMT + R2 VMT + ---- + Rn VMT}$ 

where Ri VMT = VMT of Route i,

Ri BI = Buffer Index of Route i

Also, the monthly and yearly total values of the traffic-flow MOEs for morning and afternoon routes were estimated to observe the network-wide traffic-flow trends. The rest of this chapter summarizes the monthly and yearly variation trends of the travel-time reliability and traffic-flow MOE values, estimated under all conditions, for both morning and afternoon networks from January 2018 to December 2023. Further, the yearly vulnerability trends of the individual routes are analyzed by combining the yearly estimates of the buffer index and the travel rate of each route. In addition, the effects of the geometric configuration of each route on its TTR and traffic-flow MOEs are also assessed by analyzing the relationships between the geometric friction factor, defined in the previous phase of this study, and the TTR and traffic-flow MoEs of each route. The monthly and yearly trends of the TTR and traffic-flow MOEs of each route.

# **3.2** Network-wide Traffic-Flow Performance and Travel-Time Reliability Trends

#### NETWORK-WIDE TRENDS OF TRAFFIC-FLOW MEASURES OF EFFECTIVENESS (MOE)

Figure 3.2.1 shows the monthly variations of the network-wide traffic-flow measures of effectiveness (MoE), which consisted with Vehicle-Miles-Traveled (VMT), Vehicle-Hours-Traveled (VHT) and Delayed-Vehicle-Hours (DVH), for both morning and afternoon routes. The yearly variations of the same traffic-flow MoEs are shown in Figure 3.2.2. As can be noted in this figure, both morning and afternoon VMT and VHT values substantially dropped in April 2020, when the state-wide traffic-restriction started because of COVID-19, resulting in substantial reduction of DVH. It can be also seen that all those three measures have been slowly, but continuously increasing since April 2020, however, as of December 2023, all the traffic-flow measures show significantly lower values than those of the pre-pandemic period. It is also noted that, while the overall traffic-flow patterns in the afternoon network are similar to those in the morning network, the afternoon network exhibits significantly higher DVH values than those of the morning network, which is consistent with the pre-pandemic period.



Figure 3.2.1 Network-wide Monthly Variations of Traffic-Flow Measures of Effectiveness



#### Figure 3.2.2 Network-wide Yearly Variations of Traffic-Flow MoEs for Morning and Afternoon Routes

#### NETWORK-WIDE TRENDS FOR TRAVEL-TIME RELIABILITY

Figure 3.2.3 shows the monthly variations of the network-wide, weighted average TTR values, including buffer index, planning index and travel rate, for both morning and afternoon networks. The yearly trends of these indices are shown in Figure 3.2.4. As can be seen in these figures, there is a clear difference in those TTR trends before and after April 2020, when the COVID-19 traffic restriction started. I.e., the network-wide travel-time reliability was significantly improved after April 2020, i.e., those TTR measures show substantially lower values than those of the before-pandemic period. It's also noted that, the yearly TTR values of the morning network show slowly but continuously increasing trends from 2020 to 2023, while the afternoon network in 2023 has slightly lower TTR values than those of the pre-pandemic period. Further, in 2023, the buffer index of the afternoon network in 2023, all the network-wide reliability indices for both morning and afternoon networks are significantly lower than those of the pre-pandemic period. Further, in 2023, the buffer index of the afternoon network has smaller value than those in the morning network, while both planning index and travel rate show higher values than those in the morning network. This indicates the overall higher level of congestion in the afternoon network, resulting in less fluctuations in travel times than the morning network, in 2023.

The above analysis of the network-wide trends for both Travel-Time Reliability and Traffic-Flow MoEs indicates that, as of December 2023, the traffic-flows in the metro freeway network are still in the process of recovery from COVID-19.



Figure 3.2.3 Monthly Network-wide TTR Trends for Morning and Afternoon Routes







Figure 3.2.4 Yearly Network-wide TTR Trends for Morning and Afternoon Routes

# **3.3 Yearly Trends of Travel-Time Reliability on Individual** Directional Routes

In this section, the yearly TTR trends of each directional route are analyzed with the TTR measures estimated under all operating conditions. First, to assess the overall reliability status of each route, the Vulnerability Level of a given route is determined with two travel-time TTR measures, i.e., Buffer Index (BI) and Travel Rate (TR), whose formula are shown below:

BI =  $\frac{(95th \ percentile \ Travel \ Time - Average \ Travel \ Time)}{Average \ Travel \ Time}$ TR (min/mile) =  $\frac{(95th \ percential \ Travel \ Time)}{(Route \ Length)}$ 

As indicated in the above formula, BI quantifies the variability of travel times, while TR measures the excessiveness of travel times for a given route during a given period. By plotting each route's BI and TR values in a BI—TR space, as shown in Figure 3.3.1, the combined reliability, defined as vulnerability (VI), level of each route in terms of the variability and excessiveness of travel times can be assessed and compared with those of other routes.

In this study, 5 VI levels are defined in the BI-TR space, as shown in Figure 3.3.1, and each route's VI level was determined with the BI and TR values from 2018 to 2023. Figures 3.3.2 - 3.3.13 show the yearly BI-TR relationships of the morning and afternoon routes from 2018 until 2023. Further, Tables 3.3.1 - 3.3.4 include the BI and TR values, and the resulting VI levels, of each route, from 2018 to 2023. It can be noted that the slope-steepness of the data points in the BI-TR space could indicate the overall congestion and reliability status of each period, i.e., a low slope indicates higher level of congestion with lower fluctuations in travel times than those with high slopes. For example, the BI-TR relationships of the afternoon routes generally show lower slopes, i.e., higher congestion, than those of the morning routes, consistent with the network-wide TTR and traffic-flow MoE trends discussed in the previous section. Finally, Table 3.3.5 and Figure 3.3.14 summarize the yearly variation trends of the VI levels for both morning and afternoon routes from 2018 to 2023.

As can be seen from the above figures and tables, the overall vulnerability conditions of both morning and afternoon routes exhibit clear improvements in 2020, i.e., during the pandemic period, compared to those in 2018 – 2019, while the 2021-2023 VI trends indicate the traffic conditions of the metro freeway routes have not reached the pre-pandemic level.



Level	BI (95 <sup>th</sup> %-ile)	TR (95 <sup>th</sup> %-ile)
1	BI < 0.25	< 1.0
2	0.25 ≤ BI < 0.5	$1.0 \leq TR < 2.0$
3	0.5 ≤ BI < 0.75	$2.0 \leq TR < 3.0$
4	0.75 ≤ BI < 1.0	$3.0 \leq TR < 4.0$
5	1.0 ≤ BI	4.0 ≤ TR





Figure 3.3.2 BI-TR Relationship - 2018 Morning Routes



Figure 3.3.3 BI-TR Relationship - 2019 Morning Routes



Figure 3.3.4 BI-TR Relationship - 2020 Morning Routes







Figure 3.3.6 BI-TR Relationship - 2022 Morning Routes



Figure 3.3.7 BI-TR Relationship - 2023 Morning Routes



Figure 3.3.8 BI-TR Relationships – 2018 Afternoon Routes



Figure 3.3.9 BI-TR Relationships – 2019 Afternoon Routes



Figure 3.3.10 BI-TR Relationships – 2020 Afternoon Routes



Figure 3.3.11 BI-TR Relationships – 2021 Afternoon Routes



Figure 3.3.12 BI-TR Relationships – 2022 Afternoon Routes



Figure 3.3.13 BI-TR Relationships - 2023 Afternoon Routes

2018 Morning				2019 Morning				2020 Morning			
Route	BI	TR	VI Level	Route	ВІ	TR	VI Level	Route	BI	TR	VI Level
I-394 (EB): 169 - MPLS DT	0.76	2.59	4	I-35W (NB): 135 - 494	0.83	2.61	4	I-35W (NB): 494 - MPLS DT	0.52	1.73	3
I-694 (WB): TH10 - I94	0.76	3.29	4	I-494 (WB): 194 - 135E	0.76	1.99	4	I-394 (EB): 169 - MPLS DT	0.66	1.72	3
T.H.62 (WB): Hiawatha - I35W	0.76	2.97	4	I-694 (WB): TH10 - I94	0.86	2.76	4	I-694 (NB): 194 - 36	0.73	1.84	3
I-35E (NB): 494 - SPL DT	0.76	2.3	3	I-94 (WB): 694 - SPL DT	0.9	2.95	4	I-35E (NB): 494 - SPL DT	0.39	1.49	2
I-35E (SB): 694 - SPL DT	0.76	2.07	3	T.H.100 (SB): 694 - I394	0.79	2.37	4	I-35E (SB): 694 - SPL DT	0.29	1.38	2
I-35W (NB): 135 - 494	0.76	1.86	3	T.H.36 (WB): 135E - 135W	0.78	2.81	4	I-35W (NB): 135 - 494	0.29	1.31	2
I-35W (SB): 135 - 694	0.76	1.69	3	T.H.62 (WB): Hiawatha - I35W	0.82	2.79	4	I-35W (SB): 694 - MPLS DT	0.34	1.37	2
I-494 (SB): 1694 - TH212	0.76	1.64	3	I-35E (NB): 494 - SPL DT	0.65	2.46	3	I-35W (SB): 135 - 694	0.16	1.07	2
I-494 (WB): 135e-135W	0.76	2.22	3	I-35E (NB): 135 - 494	0.52	1.4	3	I-494 (WB): 135e-135W	0.39	1.31	2
I-494 (WB): 135W-212	0.76	1.97	3	I-35E (SB): 694 - SPL DT	0.52	2.24	3	I-494 (WB): 135W-212	0.23	1.22	2
I-94 (EB): TH252 - MPLS DT	0.76	2.09	3	I-35E (SB): 135 - 694	0.54	1.43	3	I-694 (WB): 36 - I35W	0.09	1.03	2
I-94 (WB): SPL DT - MPLS DT	0.76	2.36	3	I-35W (NB): 494 - MPLS DT	0.5	2.14	3	I-694 (WB): 135W - TH10	0.10	1.06	2
I-94 (WB): 694 - SPL DT	0.76	2.46	3	I-35W (SB): 694 - MPLS DT	0.66	2.43	3	I-694 (WB): TH10 - I94	0.38	1.38	2
T.H.10 (EB): 47 - TH610	0.76	1.54	3	I-35W (SB): 135 - 694	0.72	2.28	3	I-94 (EB): MPLS DT - SPL DT	0.10	1.08	2
T.H.100 (SB): 694 - I394	0.76	2.05	3	I-394 (EB): 169 - MPLS DT	0.72	2.49	3	I-94 (EB): TH101 - I494	0.28	1.33	2
T.H.100 (SB): 1394 - 494	0.76	1.97	3	I-494 (SB): 1694 - TH212	0.56	1.76	3	I-94 (EB): TH252 - MPLS DT	0.49	1.61	2
T.H.36 (WB): 135E - 135W	0.76	2.47	3	I-494 (WB): 135e-135W	0.6	2.13	3	I-94 (WB): SPL DT - MPLS DT	0.40	1.53	2
U.S.169 (SB): TH610 - 394	0.76	2.06	3	I-494 (WB): 135W-212	0.51	1.7	3	I-94 (WB): 694 - SPL DT	0.31	1.36	2
I-35E (NB): 135 - 494	0.76	1.17	2	I-694 (NB): 194 - 36	0.55	1.63	3	I-94 (WB): 95 - 694	0.09	1.10	2
I-35E (SB): 135 - 694	0.76	1.16	2	I-694 (WB): 36 - I35W	0.64	1.87	3	T.H.10 (EB): TH610 - 694	0.26	1.16	2
I-35W (NB): 494 - MPLS DT	0.76	1.7	2	I-94 (EB): TH252 - MPLS DT	0.51	2.32	3	T.H.100 (SB): 694 - I394	0.25	1.23	2
I-35W (SB): 694 - MPLS DT	0.76	1.91	2	I-94 (WB): SPL DT - MPLS DT	0.67	2.43	3	T.H.100 (SB): 1394 - 494	0.19	1.22	2
I-394 (EB): 494 - 169	0.76	1.3	2	T.H.10 (EB): 47 - TH610	0.6	1.55	3	T.H.36 (WB): 135E - 135W	0.36	1.31	2
I-494 (WB): 194 - 135E	0.76	1.33	2	T.H.10 (EB): TH610 - 694	0.5	1.79	3	T.H.36 (WB): 1694 - 135E	0.06	1.03	2
I-694 (NB): 194 - 36	0.76	1.39	2	T.H.100 (SB): 1394 - 494	0.58	2.13	3	T.H.62 (WB): Hiawatha - I35W	0.46	1.48	2

## Table 3.3.1 Yearly BI-TR and VI Levels for Morning Routes (2018-2020)

I-694 (WB): 36 - I35W	0.76	1.46	2	T.H.36 (WB): 1694 - 135E	0.61	1.86	3	T.H.62 (WB): 135W - 1494	0.47	1.78	2
I-694 (WB): 135W - TH10	0.76	1.3	2	T.H.62 (WB): 135W - 1494	0.53	2.2	3	T.H.77 (NB): TH13 - TH62	0.11	1.02	2
I-94 (EB): MPLS DT - SPL DT	0.76	1.3	2	U.S.169 (SB): TH610 - 394	0.57	2.31	3	U.S.169 (SB): 394 - 101	0.16	1.11	2
I-94 (EB): TH101 - I494	0.76	1.52	2	I-394 (EB): 494 - 169	0.42	1.34	2	U.S.169 (SB): TH610 - 394	0.27	1.31	2
I-94 (EB): 1494 - TH252	0.76	1.22	2	I-694 (WB): 135W - TH10	0.38	1.43	2	I-35E (NB): 135 - 494	0.04	0.86	1
I-94 (WB): 95 - 694	0.76	1.37	2	I-94 (EB): MPLS DT - SPL DT	0.37	1.42	2	I-35E (SB): 135 - 694	0.07	0.85	1
T.H.10 (EB): TH610 - 694	0.76	1.67	2	I-94 (EB): TH101 - I494	0.47	1.52	2	I-394 (EB): 494 - 169	0.11	0.96	1
T.H.36 (WB): 1694 - 135E	0.76	1.52	2	I-94 (EB): 1494 - TH252	0.45	1.39	2	I-494 (SB): 1694 - TH212	0.12	0.99	1
T.H.52 (NB): TH55 - SPL DT	0.76	1.1	2	I-94 (WB): 95 - 694	0.43	1.55	2	I-494 (WB): 194 - 135E	0.08	0.99	1
T.H.610 (EB): 194 - TH10	0.76	1	2	T.H.52 (NB): TH55 - SPL DT	0.35	1.16	2	I-94 (EB): 1494 - TH252	0.09	0.93	1
T.H.62 (WB): 135W - 1494	0.76	1.98	2	T.H.610 (EB): 194 - TH10	0.26	1.15	2	T.H.10 (EB): 47 - TH610	0.09	0.94	1
T.H.77 (NB): TH13 - TH62	0.76	1.29	2	T.H.77 (NB): TH13 - TH62	0.36	1.45	2	T.H.52 (NB): TH55 - SPL DT	0.09	0.89	1
U.S.169 (SB): 394 - 101	0.76	1.5	2	U.S.169 (SB): 394 - 101	0.46	1.61	2	T.H.610 (EB): 194 - TH10	0.09	0.96	1

### Table 3.3.2 Yearly BI-TR and VI Levels for Morning Routes (2021-2023)

2021 Morning				2022 Morning		2023 Morning					
Route	ві	TR	VI Level	Route	ВІ	TR	VI Level	Route	BI	TR	VI Level
I-694 (NB): 194 - 36	0.91	2.03	4	I-694 (WB): TH10 - I94	0.96	2.54	4	I-694 (WB): TH10 - I94	1.00	2.90	5
I-694 (WB): TH10 - I94	0.61	1.69	3	I-394 (EB): 169 - MPLS DT	0.60	1.78	3	I-94 (WB): 95 - 694	0.91	2.56	4
I-94 (WB): 694 - SPL DT	0.62	1.79	3	I-494 (WB): 135e-135W	0.63	1.76	3	T.H.62 (WB): Hiawatha - I35W	0.78	2.29	4
I-35E (NB): 494 - SPL DT	0.21	1.24	2	I-94 (EB): TH101 - I494	0.54	1.61	3	I-394 (EB): 169 - MPLS DT	0.57	1.84	3
I-35E (SB): 694 - SPL DT	0.16	1.20	2	I-94 (WB): SPL DT - MPLS DT	0.64	1.92	3	I-494 (WB): 135e-135W	0.61	1.91	3
I-35W (NB): 494 - MPLS DT	0.28	1.37	2	I-94 (WB): 694 - SPL DT	0.58	1.96	3	I-94 (EB): TH101 - I494	0.60	1.82	3
I-35W (NB): 135 - 494	0.19	1.08	2	T.H.36 (WB): 135E - 135W	0.58	1.73	3	I-94 (WB): SPL DT - MPLS DT	0.55	1.94	3
I-35W (SB): 694 - MPLS DT	0.14	1.12	2	T.H.62 (WB): Hiawatha - I35W	0.65	1.86	3	I-94 (WB): 694 - SPL DT	0.65	1.96	3
I-394 (EB): 169 - MPLS DT	0.26	1.26	2	I-35E (NB): 494 - SPL DT	0.35	1.53	2	T.H.36 (WB): 135E - 135W	0.68	1.91	3
I-494 (WB): 135e-135W	0.41	1.32	2	I-35E (SB): 694 - SPL DT	0.38	1.53	2	I-35E (NB): 494 - SPL DT	0.43	1.63	2
I-494 (WB): 135W-212	0.10	1.06	2	I-35W (NB): 494 - MPLS DT	0.33	1.47	2	I-35E (NB): 135 - 494	0.25	1.08	2
I-694 (WB): 36 - I35W	0.14	1.04	2	I-35W (NB): 135 - 494	0.35	1.29	2	I-35E (SB): 694 - SPL DT	0.38	1.53	2

I-694 (WB): 135W - TH10	0.31	1.31	2	I-35W (SB): 694 - MPLS DT	0.41	1.46	2	I-35W (NB): 494 - MPLS DT	0.39	1.65	2
I-94 (EB): MPLS DT - SPL DT	0.21	1.26	2	I-35W (SB): 135 - 694	0.36	1.20	2	I-35W (NB): 135 - 494	0.26	1.21	2
I-94 (EB): TH101 - I494	0.41	1.56	2	I-494 (SB): 1694 - TH212	0.16	1.02	2	I-35W (SB): 694 - MPLS DT	0.46	1.59	2
I-94 (EB): TH252 - MPLS DT	0.29	1.37	2	I-494 (WB): 194 - 135E	0.35	1.25	2	I-35W (SB): 135 - 694	0.26	1.11	2
I-94 (WB): SPL DT - MPLS DT	0.13	1.19	2	I-494 (WB): 135W-212	0.28	1.28	2	I-394 (EB): 494 - 169	0.22	1.03	2
I-94 (WB): 95 - 694	0.07	1.05	2	I-694 (NB): 194 - 36	0.44	1.38	2	I-494 (SB): 1694 - TH212	0.25	1.11	2
T.H.10 (EB): TH610 - 694	0.25	1.14	2	I-694 (WB): 36 - I35W	0.27	1.23	2	I-494 (WB): 194 - 135E	0.20	1.07	2
T.H.100 (SB): 694 - I394	0.11	1.01	2	I-694 (WB): 135W - TH10	0.24	1.22	2	I-494 (WB): I35W-212	0.33	1.36	2
T.H.100 (SB): 1394 - 494	0.11	1.09	2	I-94 (EB): MPLS DT - SPL DT	0.23	1.30	2	I-694 (NB): 194 - 36	0.33	1.25	2
T.H.36 (WB): 135E - 135W	0.32	1.27	2	I-94 (EB): 1494 - TH252	0.18	1.01	2	I-694 (WB): 36 - I35W	0.31	1.29	2
T.H.36 (WB): 1694 - 135E	0.10	1.05	2	I-94 (EB): TH252 - MPLS DT	0.35	1.56	2	I-694 (WB): 135W - TH10	0.38	1.39	2
T.H.62 (WB): Hiawatha - I35W	0.37	1.41	2	I-94 (WB): 95 - 694	0.18	1.20	2	I-94 (EB): MPLS DT - SPL DT	0.27	1.33	2
T.H.62 (WB): 135W - 1494	0.36	1.57	2	T.H.10 (EB): 47 - TH610	0.17	1.05	2	I-94 (EB): 1494 - TH252	0.31	1.16	2
T.H.77 (NB): TH13 - TH62	0.10	1.01	2	T.H.10 (EB): TH610 - 694	0.41	1.32	2	I-94 (EB): TH252 - MPLS DT	0.33	1.61	2
U.S.169 (SB): 394 - 101	0.09	1.01	2	T.H.100 (SB): 694 - I394	0.25	1.19	2	T.H.10 (EB): 47 - TH610	0.33	1.24	2
U.S.169 (SB): TH610 - 394	0.37	1.46	2	T.H.100 (SB): 1394 - 494	0.22	1.23	2	T.H.10 (EB): TH610 - 694	0.39	1.38	2
I-35E (NB): 135 - 494	0.08	0.88	1	T.H.36 (WB): 1694 - 135E	0.27	1.25	2	T.H.100 (SB): 694 - I394	0.43	1.41	2
I-35E (SB): 135 - 694	0.06	0.83	1	T.H.62 (WB): 135W - 1494	0.49	1.92	2	T.H.100 (SB): 1394 - 494	0.27	1.28	2
I-35W (SB): 135 - 694	0.08	0.95	1	T.H.77 (NB): TH13 - TH62	0.41	1.47	2	T.H.36 (WB): 1694 - 135E	0.33	1.33	2
I-394 (EB): 494 - 169	0.06	0.89	1	U.S.169 (SB): 394 - 101	0.22	1.15	2	T.H.610 (EB): 194 - TH10	0.17	1.05	2
I-494 (SB): 1694 - TH212	0.07	0.93	1	U.S.169 (SB): TH610 - 394	0.42	1.55	2	T.H.62 (WB): 135W - 1494	0.41	1.82	2
I-494 (WB): 194 - 135E	0.06	0.93	1	I-35E (NB): 135 - 494	0.17	0.98	1	T.H.77 (NB): TH13 - TH62	0.32	1.41	2
I-94 (EB): 1494 - TH252	0.14	0.97	1	I-35E (SB): 135 - 694	0.23	0.99	1	U.S.169 (SB): 394 - 101	0.23	1.16	2
T.H.10 (EB): 47 - TH610	0.10	0.95	1	I-394 (EB): 494 - 169	0.14	0.95	1	U.S.169 (SB): TH610 - 394	0.47	1.65	2
T.H.52 (NB): TH55 - SPL DT	0.09	0.87	1	T.H.52 (NB): TH55 - SPL DT	0.12	0.91	1	I-35E (SB): 135 - 694	0.20	0.98	1
T.H.610 (EB): 194 - TH10	0.07	0.95	1	T.H.610 (EB): 194 - TH10	0.12	0.99	1	T.H.52 (NB): TH55 - SPL DT	0.18	0.98	1

Table 3.3.3	Yearly BI-T	R and VI Leve	els for Afterno	on Routes	(2018-2020)
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2018 Afternoon		2019 Afternoon				2020 Afternoon					
Route	ві	TR	VI Level	Route	BI	TR	VI Level	Route	ві	TR	VI Level
I-494 (EB): 212 - I35W	0.55	3.55	4	T.H.36 (EB):I35W - I35E	0.87	4.53	5	I-494 (EB): 212 - I35W	0.86	2.28	4
I-94 (WB):SPL DT - MPLS DT	0.61	3.09	4	I-494 (EB):135E - 194	1.25	3.23	4	I-94 (WB):I494 - TH101	0.89	2.02	4
T.H.36 (EB):I35W - I35E	0.75	3.28	4	I-494 (EB): 212 - I35W	0.52	3.47	4	T.H.62 (EB):1494 - 135W	0.75	2.44	4
T.H.62 (EB):1494 - 135W	0.51	3.67	4	I-694 (EB): i94 - TH10	0.68	3.12	4	I-35E (NB): SPL DT - 694	0.56	1.59	3
I-35E (NB): SPL DT - 694	0.45	2.04	3	I-94 (WB):SPL DT - MPLS DT	0.56	3.01	4	I-35W (SB): MPLS DT - 494	0.60	1.79	3
I-35E (SB): SPL DT - 494	0.57	2.35	3	T.H.100 (NB): 494 - I394	0.86	3.22	4	I-694 (EB): i94 - TH10	0.60	1.59	3
I-35W (NB): MPLS DT - 694	0.46	2.05	3	T.H.62 (EB):1494 - 135W	0.45	3.39	4	I-94 (EB): MPLS DT - SPL DT	0.71	1.88	3
I-35W (SB): MPLS DT - 494	0.37	2.07	3	T.H.77 (SB):TH62 - TH13	0.76	2.17	4	I-94 (WB):SPL DT - MPLS DT	0.68	1.96	3
I-494 (EB):135E - 194	0.58	1.75	3	I-35E (NB): SPL DT - 694	0.45	2.18	3	T.H.36 (EB):135W - 135E	0.67	1.81	3
I-694 (EB): I35W - 36	0.59	1.63	3	I-35E (SB): SPL DT - 494	0.68	2.55	3	U.S.169 (NB): 101 - 394	0.52	1.45	3
I-694 (EB): i94 - TH10	0.62	2.44	3	I-35W (NB): MPLS DT - 694	0.66	2.69	3	U.S.169 (NB): 394 - TH610	0.51	1.67	3
I-694 (SB): 36 - i94	0.74	1.75	3	I-35W (SB): MPLS DT - 494	0.50	2.60	3	I-35E (SB): 494 - 135	0.21	1.03	2
I-94 (EB): MPLS DT - SPL DT	0.58	2.59	3	I-494 (NB):TH212 - I694	0.55	2.09	3	I-35E (SB): SPL DT - 494	0.35	1.38	2
1394 (WB): MPLS DT - 169	0.48	2.15	3	I-694 (EB): I35W - 36	0.72	1.73	3	I-35W (NB): 694 - 135	0.43	1.37	2
T.H.10 (WB): 694 - TH610	0.32	2.15	3	I-694 (EB): TH10 - I35W	0.75	2.42	3	I-35W (NB): MPLS DT - 694	0.38	1.41	2
T.H.100 (NB): 494 - I394	0.72	2.56	3	I-694 (SB): 36 - i94	0.69	1.64	3	I-35W (SB): 494 - 135	0.41	1.39	2
T.H.100 (NB): I394 - 694	0.56	2.44	3	I-94 (EB): MPLS DT - SPL DT	0.59	2.88	3	I-494 (EB):135E - 194	0.26	1.08	2
T.H.77 (SB):TH62 - TH13	0.56	1.61	3	I-94 (EB): SPL DT - 694	0.50	1.93	3	I-494 (EB): I35W-I35e	0.46	1.47	2
U.S.169 (NB): 101 - 394	0.54	1.87	3	I-94 (WB):I494 - TH101	0.60	2.00	3	I-494 (NB):TH212 - I694	0.35	1.23	2
U.S.169 (NB): 394 - TH610	0.47	2.10	3	1394 (WB): MPLS DT - 169	0.48	2.04	3	I-694 (EB): I35W - 36	0.24	1.04	2
I-35E (NB): 694 - 135	0.28	1.05	2	T.H.10 (WB): 694 - TH610	0.32	2.01	3	I-694 (EB): TH10 - I35W	0.43	1.33	2
I-35E (SB): 494 - 135	0.30	1.25	2	T.H.100 (NB): I394 - 694	0.51	2.24	3	I-694 (SB): 36 - i94	0.25	1.05	2
I-35W (NB): 694 - 135	0.21	1.24	2	T.H.62 (EB):I35W - HiaWatha	0.53	2.41	3	I-94 (EB): 694 - 95	0.14	1.06	2
I-35W (SB): 494 - 135	0.45	1.55	2	U.S.169 (NB): 101 - 394	0.60	2.18	3	I-94 (EB): SPL DT - 694	0.35	1.30	2
I-494 (EB): I35W-I35e	0.29	1.50	2	U.S.169 (NB): 394 - TH610	0.36	2.03	3	I-94 (WB): MPLS DT - TH252	0.14	1.13	2
I-494 (NB):TH212 - I694	0.49	1.82	2	I-35E (NB): 694 - 135	0.37	1.18	2	1394 (WB): 169 - 494	0.28	1.15	2
I-694 (EB): TH10 - I35W	0.48	1.47	2	I-35E (SB): 494 - 135	0.35	1.33	2	1394 (WB): MPLS DT - 169	0.41	1.41	2

I-94 (EB): 694 - 95	0.29	1.35	2	I-35W (NB): 694 - 135	0.39	1.52	2	T.H.10 (WB): 694 - TH610	0.50	1.60	2
I-94 (EB): SPL DT - 694	0.41	1.59	2	I-35W (SB): 494 - 135	0.40	1.80	2	T.H.10 (WB): TH610 - 47	0.45	1.40	2
I-94 (WB):I494 - TH101	0.44	1.67	2	I-494 (EB): I35W-I35e	0.38	1.68	2	T.H.100 (NB): 494 - 1394	0.49	1.49	2
I-94 (WB): MPLS DT - TH252	0.36	1.56	2	I-94 (EB): 694 - 95	0.31	1.28	2	T.H.100 (NB): 1394 - 694	0.43	1.45	2
I-94 (WB): TH252 - I494	0.26	1.14	2	I-94 (WB): MPLS DT - TH252	0.40	1.62	2	T.H.36 (EB):135E - 1694	0.18	1.20	2
I394 (WB): 169 - 494	0.22	1.26	2	I-94 (WB): TH252 - I494	0.42	1.32	2	T.H.62 (EB):I35W - HiaWatha	0.37	1.57	2
T.H.10 (WB): TH610 - 47	0.32	1.53	2	I394 (WB): 169 - 494	0.27	1.35	2	T.H.77 (SB):TH62 - TH13	0.21	1.10	2
T.H.36 (EB):135E - 1694	0.25	1.45	2	T.H.10 (WB): TH610 - 47	0.41	1.61	2	I-35E (NB): 694 - 135	0.13	0.90	1
T.H.610 (WB):TH10 - I94	0.37	1.31	2	T.H.36 (EB):135E - 1694	0.32	1.55	2	I-94 (WB): TH252 - I494	0.11	0.90	1
T.H.62 (EB):I35W - HiaWatha	0.32	1.97	2	T.H.610 (WB):TH10 - I94	0.28	1.22	2	T.H.52 (SB):SPL DT - TH55	0.07	0.84	1
T.H.52 (SB):SPL DT - TH55	0.17	0.96	1	T.H.52 (SB):SPL DT - TH55	0.17	0.97	1	T.H.610 (WB):TH10 - I94	0.12	1.00	1

#### Table 3.3.4 Yearly BI-TR and VI Levels for Afternoon Routes (2021-2023)

2021 Afternoon	2022 Afternoon	2023 Afternoon									
Route	BI	TR	VI Level	Route	BI	TR	VI Level	Route	BI	TR	VI Level
T.H.10 (WB): 694 - TH610	1.51	3.35	5	I-94 (EB): 694 - 95	1.60	3.46	5	I-494 (EB): 212 - I35W	0.59	3.36	4
I-494 (EB): 212 - I35W	0.54	2.61	3	I-494 (EB):135E - 194	0.99	1.88	4	I-694 (EB): i94 - TH10	0.61	2.51	3
I-694 (EB): i94 - TH10	0.72	2.27	3	I-494 (EB): 212 - I35W	0.67	3.36	4	I-94 (EB): MPLS DT - SPL DT	0.52	2.17	3
I-94 (EB): MPLS DT - SPL DT	0.56	2.00	3	I-35E (SB): SPL DT - 494	0.64	1.97	3	I-94 (EB): 694 - 95	0.96	2.76	3
I-94 (WB):I494 - TH101	0.78	2.09	3	I-694 (EB): I35W - 36	0.73	1.84	3	I-94 (WB):SPL DT - MPLS DT	0.53	2.73	3
I-94 (WB):SPL DT - MPLS DT	0.59	2.14	3	I-694 (EB): i94 - TH10	0.72	2.59	3	T.H.100 (NB): 494 - I394	0.71	1.98	3
T.H.100 (NB): I394 - 694	0.58	1.77	3	I-94 (EB): MPLS DT - SPL DT	0.59	2.25	3	T.H.100 (NB): I394 - 694	0.51	2.03	3
T.H.36 (EB):I35W - I35E	0.64	2.05	3	I-94 (WB):SPL DT - MPLS DT	0.72	2.88	3	T.H.36 (EB):I35W - I35E	0.55	2.12	3
T.H.62 (EB):1494 - 135W	0.52	2.64	3	T.H.10 (WB): 694 - TH610	0.51	1.54	3	T.H.62 (EB):1494 - 135W	0.46	3.00	3
U.S.169 (NB): 394 - TH610	0.52	1.80	3	T.H.100 (NB): I394 - 694	0.57	2.05	3	I-35E (NB): SPL DT - 694	0.43	1.69	2
I-35E (NB): SPL DT - 694	0.31	1.33	2	T.H.36 (EB):135W - 135E	0.68	2.27	3	I-35E (SB): 494 - 135	0.19	1.04	2
I-35E (SB): SPL DT - 494	0.32	1.43	2	T.H.62 (EB):1494 - 135W	0.51	2.76	3	I-35E (SB): SPL DT - 494	0.45	1.80	2
I-35W (NB): 694 - 135	0.34	1.21	2	I-35E (NB): SPL DT - 694	0.45	1.57	2	I-35W (NB): MPLS DT - 694	0.31	1.36	2
I-35W (NB): MPLS DT - 694	0.50	1.62	2	I-35E (SB): 494 - 135	0.23	1.08	2	I-35W (SB): 494 - 135	0.26	1.18	2
I-35W (SB): MPLS DT - 494	0.41	1.73	2	I-35W (NB): 694 - 135	0.43	1.27	2	I-35W (SB): MPLS DT - 494	0.42	1.76	2
I-494 (EB): I35W-I35e	0.21	1.15	2	I-35W (NB): MPLS DT - 694	0.38	1.39	2	I-494 (EB):I35E - I94	0.29	1.09	2
I-494 (NB):TH212 - I694	0.32	1.19	2	I-35W (SB): 494 - 135	0.38	1.32	2	I-494 (EB): I35W-I35e	0.2	1.22	2

I-694 (EB): I35W - 36	0.37	1.21	2	I-35W (SB): MPLS DT - 494	0.41	1.69	2	I-494 (NB):TH212 - I694	0.39	1.44	2
I-694 (EB): TH10 - I35W	0.40	1.36	2	I-494 (EB): I35W-I35e	0.40	1.45	2	I-694 (EB): I35W - 36	0.46	1.41	2
I-694 (SB): 36 - i94	0.19	1.02	2	I-494 (NB):TH212 - I694	0.39	1.36	2	I-694 (EB): TH10 - I35W	0.39	1.44	2
I-94 (EB): 694 - 95	0.10	1.03	2	I-694 (EB): TH10 - I35W	0.49	1.52	2	I-694 (SB): 36 - i94	0.17	1.02	2
I-94 (EB): SPL DT - 694	0.11	1.03	2	I-694 (SB): 36 - i94	0.28	1.16	2	I-94 (EB): SPL DT - 694	0.27	1.28	2
I-94 (WB): MPLS DT - TH252	0.17	1.21	2	I-94 (EB): SPL DT - 694	0.44	1.57	2	I-94 (WB):I494 - TH101	0.39	1.34	2
1394 (WB): MPLS DT - 169	0.31	1.31	2	I-94 (WB):I494 - TH101	0.33	1.20	2	I-94 (WB): MPLS DT - TH252	0.14	1.29	2
T.H.10 (WB): TH610 - 47	0.55	1.61	2	I-94 (WB): MPLS DT - TH252	0.19	1.33	2	I-94 (WB): TH252 - I494	0.25	1.12	2
T.H.100 (NB): 494 - I394	0.32	1.30	2	I-94 (WB): TH252 - I494	0.23	1.06	2	I394 (WB): 169 - 494	0.19	1.04	2
T.H.36 (EB):135E - 1694	0.14	1.18	2	I394 (WB): 169 - 494	0.20	1.03	2	1394 (WB): MPLS DT - 169	0.36	1.59	2
T.H.610 (WB):TH10 - I94	0.18	1.05	2	1394 (WB): MPLS DT - 169	0.38	1.59	2	T.H.10 (WB): 694 - TH610	0.41	1.48	2
T.H.62 (EB):I35W - HiaWatha	0.30	1.67	2	T.H.10 (WB): TH610 - 47	0.46	1.64	2	T.H.10 (WB): TH610 - 47	0.41	1.59	2
U.S.169 (NB): 101 - 394	0.36	1.28	2	T.H.100 (NB): 494 - I394	0.48	1.52	2	T.H.36 (EB):135E - 1694	0.27	1.40	2
I-35E (NB): 694 - 135	0.10	0.87	1	T.H.36 (EB):135E - 1694	0.43	1.65	2	T.H.610 (WB):TH10 - I94	0.44	1.47	2
I-35E (SB): 494 - 135	0.10	0.93	1	T.H.610 (WB):TH10 - I94	0.29	1.19	2	T.H.62 (EB):I35W - HiaWatha	0.25	1.84	2
I-35W (SB): 494 - 135	0.14	0.97	1	T.H.62 (EB):I35W - HiaWatha	0.31	1.89	2	T.H.77 (SB):TH62 - TH13	0.15	1.08	2
I-494 (EB):135E - 194	0.19	0.97	1	T.H.77 (SB):TH62 - TH13	0.49	1.52	2	U.S.169 (NB): 101 - 394	0.41	1.47	2
I-94 (WB): TH252 - I494	0.14	0.94	1	U.S.169 (NB): 101 - 394	0.39	1.38	2	U.S.169 (NB): 394 - TH610	0.42	1.67	2
1394 (WB): 169 - 494	0.09	0.93	1	U.S.169 (NB): 394 - TH610	0.40	1.62	2	I-35E (NB): 694 - 135	0.07	0.83	1
T.H.52 (SB):SPL DT - TH55	0.06	0.84	1	I-35E (NB): 694 - 135	0.21	0.96	1	I-35W (NB): 694 - 135	0.15	0.98	1
T.H.77 (SB):TH62 - TH13	0.09	0.97	1	T.H.52 (SB):SPL DT - TH55	0.15	0.91	1	T.H.52 (SB):SPL DT - TH55	0.07	0.85	1

Table 3.3.5 Yearly Variations of Vulnerability Levels for Morning and Afternoon Routes

Morning							Afternoon						
	2018	2019	2020	2021	2022	2023		2018	2019	2020	2021	2022	2023
level 1	0	0	9	10	5	2	level 1	1	1	4	8	2	3
level 2	20	10	26	25	25	27	level 2	17	12	23	20	24	26
level 3	15	21	3	2	7	6	level 3	16	17	8	9	9	8
level 4	3	7	0	1	1	2	level 4	4	7	3	0	2	1
level 5	0	0	0	0	0	1	level 5	0	1	0	1	1	0



Figure 3.3.14 Yearly Variations of Vulnerability Levels (Morning/Afternoon Routes)

# **3.4 Effects of Geometric Configuration on Travel-Time Reliability and Traffic-Flow Measures**

In this section, the potential effects of the route-wide geometric configuration on the travel-time reliability (TTR) and traffic-flow measures are analyzed. The previous phase of this study modeled the 'strength' of the route-wide geometric structure, G, in terms of handling the traffic flows going through a given corridor as follows:

$$G = \frac{G1 * G3 * G4}{G2}$$

where, G: Geometric Friction Index of a given route,

- G1: Number of Exit Ramps per mile (G1) in a given route,
- G2: Number of Entrance Ramps per mile (G2) for a given route,
- G3: Proportion of the total non-weaving section length in a given route,
  - =  $1 [\Sigma (\text{lengths of all weaving sections}) / \text{route length}],$
- G4: Average number of through lanes for a given route weighted with the distance from upstream

#### boundary.

In the above formula, the numerator reflects the combined effects of the geometric features facilitating through movements, while the denominator represents the potential interruption caused by the entering volumes to the route-wide, through-traffic flows. Therefore, the routes with high G values can be considered to have less friction to through-traffic flows than those with low G values.

In this study, the G values of all 74 directional routes were calculated with the geometry values extracted from the Google-earth map. Further, the potential effects of the geometric friction on TTR are analyzed by examining the relationships between G and the yearly TTR values, i.e., Buffer Index (BI) and Planning Index (PI), of each route. It can be expected that the routes with less geometric friction (high G values), have generally better TTR, i.e., low values of BI and PI resulting from less fluctuations in travel times with lower levels of congestion, than those with high friction (low G values).

Figures 3.4.1 - 3.4.24 show the yearly variations of G - BI and G - PI relationships estimated for both morning and afternoon routes from 2018 to 2023. As noted from these figures, the G - BI and G - PI patterns of the metro freeways generally follow the expected relationships, i.e., as G value increases, both BI and PI values tend to decrease. It can be also noted that there are multiple route groups with slightly different G-BI/PI slopes, indicating the need for further enhancement of the G model. However, no notable pattern is observed for both morning and afternoon routes in 2020, when there was substantial reduction in traffic flows because of the pandemic.

The effects of the geometric configuration on the traffic-flow measures of effectiveness, i.e., Vehicle-Miles-Traveled (VMT) and Vehicle-Hours-Traveled (VHT), are shown in Figures 3.4.25 - and 3.4.48, which include the yearly variations of G-VMT/mile and G-VHT/mile relationships for both morning and afternoon routes. As expected, the routes with high G values, i.e., low levels of friction to through traffic, exhibit higher values of VMT/mile and VHT/mile than those with low G values.

The above observations can provide the basis for future geometric changes of the freeway corridors in terms of improving the efficiency of traffic flows and travel-time reliability.

#### Buffer Index (BI) vs Geometric-Friction Index (G) - 2018 - 2023: Morning Routes



Figure 3.4.1 BI – G Relationship (2018 Morning Routes)



Figure 3.4.2 BI – G Relationship (2019 Morning Routes)



Figure 3.4.3 BI – G Relationship (2020 Morning Routes)



Figure 3.4.4 BI – G Relationship (2021 Morning Routes)



Figure 3.4.5 BI – G Relationship (2022 Morning Routes)



Figure 3.4.6 BI – G Relationship (2023 Morning Routes)

Buffer Index vs Geometric Friction Index (2018 – 2023: Afternoon Routes)


Figure 3.4.7 BI – G Relationship (2018 Afternoon Routes)



Figure 3.4.8 BI – G Relationship (2019 Afternoon Routes)



Figure 3.4.9 BI – G Relationship (2020 Afternoon Routes)



Figure 3.4.10 BI – G Relationship (2021 Afternoon Routes)



Figure 3.4.11 BI – G Relationship (2022 Afternoon Routes)



Figure 3.4.12 BI - G Relationship (2023 Afternoon Routes)

Planning Index (PI) vs Geometric Friction Index (G) - 2018 to 2023: Morning Routes







Figure 3.4.14 PI - G Relationship (2019 Morning Routes)



Figure 3.4.15 PI - G Relationship (2020 Morning Routes)



Figure 3.4.16 PI - G Relationship (2021 Morning Routes)



Figure 3.4.17 PI - G Relationship (2022 Morning Routes)



Figure 3.4.18 PI - G Relationship (2023 Morning Routes)

Planning Index (PI) vs Geometric Friction Index (G) - 2018 to 2023: Afternoon Routes



Figure 3.4.19 PI - G Relationship (2018 Afternoon Routes)







Figure 3.4.21 PI - G Relationship (2020 Afternoon Routes)



Figure 3.4.22 PI - G Relationship (2021 Afternoon Routes)



Figure 3.4.23 PI - G Relationship (2022 Afternoon Routes)



Figure 3.4.24 PI - G Relationship (2023 Afternoon Routes)

Vehicle-Miles-Traveled (VMT)mile vs Geometric-Friction Index (G) - 2018 – 2023: Morning Routes



Figure 3.4.25 VMT/mile - G Relationship (2018 Morning Routes)



Figure 3.4.26 VMT/mile - G Relationship (2019 Morning Routes)



Figure 3.4.27 VMT/mile - G Relationship (2020 Morning Routes)



Figure 3.4.28 VMT/mile - G Relationship (2021 Morning Routes)



Figure 3.4.29 VMT/mile - G Relationship (2022 Morning Routes)



Figure 3.4.30 VMT/mile - G Relationship (2023 Morning Routes)

Vehicle-Miles-Traveled (VMT)/mile vs Geometric-Friction Index (G) - 2018 – 2023: Afternoon Routes



Figure 3.4.31 VMT/mile - G Relationship (2018 Afternoon Routes)



Figure 3.4.32 VMT/mile - G Relationship (2019 Afternoon Routes)



Figure 3.4.33 VMT/mile - G Relationship (2020 Afternoon Routes)



Figure 3.4.34 VMT/mile - G Relationship (2021 Afternoon Routes)



Figure 3.4.35 VMT/mile - G Relationship (2022 Afternoon Routes)



Figure 3.4.36 VMT/mile - G Relationship (2023 Afternoon Routes)

Vehicle-Hours-Traveled (VHT)/mile vs Geometric-Friction Index (G) - 2018-2023: Morning Routes



Figure 3.4.37 VHT/mile - G Relationship (2018 Morning Routes)



Figure 3.4.38 VHT/mile - G Relationship (2019 Morning Routes)



Figure 3.4.39 VHT/mile - G Relationship (2020 Morning Routes)



Figure 3.4.40 VHT/mile - G Relationship (2021 Morning Routes)



Figure 3.4.41 VHT/mile - G Relationship (2022 Morning Routes)



Figure 3.4.42 VHT/mile - G Relationship (2023 Morning Routes)

Vehicle-Hours-Traveled (VHT)/mile vs Geometric-Friction Index (G) - 2018-2023: Afternoon Routes



Figure 3.4.43 VHT/mile - G Relationship (2018 Afternoon Routes)



Figure 3.4.44 VHT/mile - G Relationship (2019 Afternoon Routes)



Figure 3.4.45 VHT/mile - G Relationship (2020 Afternoon Routes)


Figure 3.4.46 VHT/mile - G Relationship (2021 Afternoon Routes)



Figure 3.4.47 VHT/mile - G Relationship (2022 Afternoon Routes)



Figure 3.4.48 VHT/mile - G Relationship (2023 Afternoon Routes)

# 3.5 Monthly and Yearly Trends of Travel-Time Reliability and Traffic-Flow Measures at Individual

#### ROUTES

As described earlier, this study also analyzed the monthly and yearly trends of individual travel-time reliability (TTR) and traffic-flow measures of effectiveness (MoE) from January 2018 until December 2023. In this section, a sample output for such an individual-route analysis for the 169 northbound (NB) route is presented. Figures 3.5.1 - 3.5.9 show the monthly and yearly variations of the TTR and traffic-flow MoE measures at the example route. The results of all other routes are included in the Appendix.

### MONTHLY RELIABILITY TRENDS OF US-169 NB ROUTE (TH 101 < -- > I-394) FOR AFTERNOON-PEAK PERIOD



#### Figure 3.5.1 Location of 169 NB Route



#### Effects of Weather conditions on Travel-Time Reliability (TTR)



Effects of Incidents



Figure 3.5.3 Monthly Variations of TTR Measures under Different Incident Conditions (169 NB)

Effects of Work Zones



Figure 3.5.4 Monthly Variations of TTR Measures under Different Work-Zone Conditions (169 NB)

Yearly Variations of TTR - Weather Effects



Figure 3.5.5 Yearly Variations of TTR Measures under Different Weather Conditions (169 NB)

Incident Effects



Figure 3.5.6 Yearly Variations of TTR Measures under Different Incident Conditions (169 NB)





Figure 3.5.7 Yearly Variations of TTR Measures under Different Work-Zone Conditions (169 NB)

Yearly Variations of Buffer Index - Travel Rate Relationships



Figure 3.5.8 Yearly Variations of Buffer-Index vs Travel-Rate Relationships (169 NB)

Variations of Traffic-Flow Measures



Figure 3.5.9 Yearly Variations of Traffic-Flow Measures (169 NB)

#### **Trends Summary for 169 NB Route**

- The congestion on this route had been increasing before the pandemic as shown in the Delayed-Vehicle-Hour (DVH) and Buffer/Planning Index trends for the 2018-2019 period.
- The pandemic-induced traffic restriction in 2020 resulted in significant reduction in VMT and improvements in travel-time TTR measures. While the total DVH has been continuously increasing since 2020, the DVH in December 2023 is still significantly lower than the pre-pandemic level.
- While weather is the major factor affecting travel-time reliability, the effects of snow are significantly larger than those of rain on this route with high levels of traffic flow.

# 3.6 Summary

This chapter summarizes the analysis results for the travel-time reliability (TTR) and traffic-flow performance trends of the metro freeway network, which consists of 37 corridors and 74 directional routes. First, for each directional route, a set of the monthly/yearly TTR indices and traffic-flow measures of effectiveness (MoE) for morning and afternoon peak periods were estimated from 2018 until 2023 under various operating conditions. Next, the results from the individual-route analysis were used to determine the network-wide trends for the TTR and traffic-flow MoEs before and after April 2020, when the COVID-19 traffic-restriction started. The trends analysis showed that, after the sudden reduction in traffic flows in April 2020, the traffic flows in the metro freeway network have been slowly but continuously increasing, however, as of December 2023, they have not reached the pre-pandemic level. As a result, the network-wide TTR measures after April 2020 showed continuously better reliability conditions than those of the pre-pandemic period in both morning and afternoon networks.

The results from the individual route analysis were also used to determine the overall reliability conditions, defined as vulnerability (VI), of each route and their yearly trends. Specifically, the yearly values of the buffer index and travel rate are used to determine the vulnerability level of each route, and the most vulnerable routes were identified each year from 2018 until 2023. Further, the effects of the route-wide geometric configuration, quantified as the geometric friction factor, G, on the TTR and traffic-flow measures of each route were also analyzed by examining the graphical relationships between G and TTR/traffic-flow MoEs, whose yearly plots exhibited the expected patterns, i.e., the routes with low friction handled higher traffic flows with less fluctuations in travel times than those with high friction geometry.

The findings in this chapter provide the overall traffic conditions and trends of the metro freeway network as well as those of individual routes. Further, the analysis results of route vulnerability and the geometric effects on both TTR and traffic-flow measures could provide the basis for future improvements of freeway corridors.

# Chapter 4: ENHANCEMENT OF THE OPERATIONAL-RESILIENCE MODEL FOR FREEWAY CORRIDORS

# 4.1 Introduction and Overview of Preliminary Resilience Model

This chapter summarizes the process to enhance the preliminary model, developed in the previous phase, to quantify the corridor-wide operational resilience, which is defined as the inherent capability of a freeway traffic system to maintain its functionality by absorbing various levels of traffic disturbances and to recover from congestion with maximum efficiency under given geometry and operating conditions. Figure 4.1.1 shows the conceptual framework, described in the previous phase, of the corridor-wide traffic process, where the time-variant congestion levels, e.g., delayed-vehicle-hours, can be considered as the results from the interaction between the corridor-wide resilience and external disturbances to a given system.



Figure 4.1.1 Conceptual Framework of Corridor-wide Traffic Process

Based on the above conceptual relationship, a preliminary model to quantify the operational resilience of a given freeway directional corridor was developed in the previous phase as follows:

$$CORI_{i} = \frac{\sum_{t} (DVH_{t} * A_{t})}{(\sum_{t} V_{E,t}) * \sigma}$$

where, CORI<sub>i</sub> = Operational Resilience Index of Corridor i

DVH<sub>t</sub> = Corridor-wide delayed-vehicle-hours during t,

=  $\Sigma [(TT_{i,t} - FF_TT_i) * K_{i,t} * L_i]$  for all segment *i* in a given route,

where,  $TT_{i,t}$  = Estimated travel time of segment i during t,

*FF\_TT<sub>i</sub>* = *Free-Flow travel time of segment i, estimated with the speed limit at a given roadway section,* 

 $K_{i,t}$  = Traffic density of segment i during t,  $L_i$  = Length of segment i.

*V<sub>E,t</sub>* = Corridor-wide total entering volume during *t*,

= (Upstream Boundary Station Volume)<sub>t</sub> +  $\Sigma$  (All Entrance Ramp Volumes)<sub>t</sub>

 $\sigma$  = Standard deviation of  $V_{E,t}$  during a peak period.

A<sub>t</sub> = Proportion of weighted average number of through lanes during t,

 $=\frac{Average Weighted Number of Through Lanes - (Number of Lanes Blocked)_t}{Average Weighted Number of Through Lanes}$ 

As noted from the above formulation, the preliminary model tries to quantify the operational resilience of a given directional corridor as its capability to minimize delay under given time-variant traffic demand and through capacity, i.e., a smaller CORI index indicates a stronger resilience level for a given corridor. It also needs to be noted that, in the preliminary modeling, the *'Average Weighted-Number of Through Lanes'* is used as a surrogate measure of the through capacity for a given route. In particular, a set of the assumed values for blocked lanes, derived from the historical data, were used to reflect the incident effects on the through capacity.

The preliminary model was tested with the traffic data collected from a total of 8 sample routes for the period of September-October 2019 and the daily estimates of the Corridor-wide Operational Resilience Index (CORI) values for each route were calculated for the dry days. Figure 4.1.3 shows the locations of those sample routes. The daily estimates of CORI values for each sample route showed relatively consistent values as reported in the previous phase (2). Further, the potential relationship between the corridor-wide operational resilience and the geometric structure of each corridor was also studied in the previous phase by examining the relationships between the 'Geometric-Friction Index', G, described in the previous chapter as the quantification of the 'strength' of the geometric structure of a given corridor in terms of facilitating through traffic movements. Figure 4.1.3 shows the relationship between Geometric Friction Factor and CORI, Corridor-wide Operational Resilience Index, estimated with the preliminary model for the sample routes.



Figure 4.1.2 Locations of the Sample Routes (5 Corridors/8 Directional Routes)



Figure 4.1.3 Operational Resilience (Preliminary Model) vs G for Sample Routes

# **4.2 Enhancements of the Preliminary Model for Operational Resilience of Freeway Corridors**

As described in the previous section, the testing results of the preliminary model with the data collected from the sample routes showed relatively consistent daily estimates of CORI values for each sample route. Further, it also showed promising possibilities in quantifying the operational resilience of a given corridor traffic system as a function of its geometric configuration. However, the following issues were identified for the preliminary resilience model to be applied for field operations:

- The preliminary model is based on the Delayed-Vehicle-Hours (DVH) as the system performance measure. While DVH through time can reflect the interaction between the external input to a corridortraffic system and its operational resilience, the values of DVH through time can vary depending on the number of lanes and length of a given route, i.e., DVH values for a given route are not normalized and cannot be used for direct comparison with other routes.
- In the preliminary model, a set of assumed values were used for the number of blocked lanes for different types of accident. However, it has been found that those values can vary for the same type of accident depending on the traffic level and surrounding conditions, e.g., the effects of a shoulder-block incident on the light traffic flows may not be the same as those on the medium to heavy traffic flows at a same location. Therefore, applying a set of the fixed values of blocked lanes may not correctly capture the effects of incidents on the resilience.
- It has been noted that most freeway routes in the current metro network contain some entrance ramps with missing detector data. The lack of some entrance-ramp data directly affects the accuracy of the 'total entering-volume through time', an important parameter in the preliminary model. Further, those missing ramp-flow rates could not be estimated correctly through imputation if adjacent detector data are not available.

To address the above issues, the following enhancements have been made to the preliminary model:

First, the route-wide Space-Mean Speed, U<sub>t</sub>, is used as the main performance measure, which is estimated through time using only the mainline detector-station data as follows:

 $U_t = (VMT)_t / (VHT)_t$ 

where, Ut = Route-wide space-mean speed (mph) during t,

(VMT)<sub>t</sub> = Route-wide Vehicle-Miles-Traveled during t,

 $= \Sigma (Q_{i,t} * T * L_i)$  for all i, t

where, Q<sub>i,t</sub>: Flow-Rate of Segment i during t, T: Time duration of t, L<sub>i</sub>: Length of Segment, i

(VHT)<sub>t</sub> = Route-wide Vehicle-Hours-Traveled during t

=  $\Sigma$  ( $K_{i,t} * L_i * t_i$ ) for all  $L_i$ ,  $t_i$  where,  $K_{i,t}$ : Density of Segment i, during t

In the above formulation, the Flow Rate and Density of Segment i through time are estimated with the traffic data from the field detector stations on the mainline in each route. Figure 4.2.1 shows the  $U_t$  variations through time at the I-494 NB and the 100 NB routes, two of the sample routes used in this study, for the same weekdays. As indicated in this figure, the  $U_t$ -time variations on each weekday clearly exhibit the route-wide resilience patterns in terms of resistance/adaptation and recovery periods. It can also be noted that the route-wide space-mean speed is a normalized measure that can be directly used for comparing the traffic performance of multiple routes.





Next, a new route-wide measure, *Total Number of Mainline Vehicles on a Given Route during t*,  $V_t$ , is added to the modeling.  $V_t$  captures the combined effects of the interaction among all the entrance/exit ramp volumes and the mainline volume during t on a given route, i.e.,

 $V_t = \Sigma (K_{i,t} * L_i)$  for all segment *i*,  $L_i$ : Length of segment *i* 

Figure 4.2.2 shows the variations of  $V_t$  and  $U_t$  on a weekday at the I-494 NB route, which had a crash accident from 16:55 until 18:00. As indicated in this figure, Ut directly responds to the variations of  $V_t$ . In particular, the  $V_t$ - $U_t$  variations during the crash period shows that the  $V_t$  variations can directly reflect the effects of an incident, i.e., the resilience of a given route responding to an incident could be captured by the interrelationship between  $V_t$  and  $U_t$ .



#### Figure 4.2.2 Vt and Ut Variations through time at I-494 NB Route

Based on the above analysis, two alternative models are developed in this study as the potential enhancements of the preliminary model to quantify the operational resilience of freeway corridors and tested with the same set of data used in the previous phase as follows:

Model 1: 
$$CORI_i = \frac{\sum_{t,i} (\Delta U_t * t_i) * \sum_t (V_t)}{\sum_t (E_t)}$$

where CORI: Corridor-wide (directional) Operational Resilience Index

 $\Delta U_t$ :  $U_f - U_t$ , where  $U_f$ : Free-Flow Speed (estimated with the speed limit at a given location),

*t<sub>i</sub>*: Duration of time interval: *i* => 0 – *T*, *T*: Total Time Period,

 $\Sigma V_t$ : Total Number of Vehicles on the Mainline during t,

*E<sub>t</sub>* : Total Route-wide Entering Volume during *t*,

Model 2:  $CORI_i = \frac{\sum_{t,i} (\Delta U_t * t_i)}{(\sum_t v_t)}$ 

where  $v_t$ : Total Number of Vehicles on the Mainline per mile during t, i.e.,  $V_t$  / (Route Length)

In the above formulations,  $(\Delta U_t * t_i)$  quantifies the functional loss of the route-wide traffic system during *t*. In *Model 1 (M1)*, the operational resilience of a directional corridor is measured as the total route-wide functional loss per entering vehicle, while *Model 2 (M2)* determines the total functional loss per mainline vehicle as a measure of the CORI for a given route. In both models, the smaller value of CORI indicates stronger resilience for a given corridor traffic system. It can also be noted that in both models the number of blocked lanes because of an incident is not included, i.e., the resilience of a given route responding to an incident is designed to be captured through the interaction between  $V_t$  and  $U_t$ .

In this study, both *M1* and *M2* models are tested with the same set of data, i.e., the traffic data for the dry weekdays in September-October 2019, used in the previous phase for developing the preliminary model. Figures 4.2.3 and 4.2.4 show the daily CORI estimates for the sample routes for the same days with *M1* and *M2*. The daily CORI estimates with *M2* are also compared with those from the preliminary model as shown in Figure 4.2.5. As noted from these figures, the CORI estimates with *M2* exhibit significantly consistent daily values, i.e., substantially less day-to-day variations, than those from either the preliminary model or M1 for both afternoon and morning peak routes. It can be noted that M2 does not include the *'Total Entering Volume, Et'*, which has potential accuracy issues depending on the availability of entrance ramp data.

Figure 4.2.6 shows the relationships between the Geometric Friction Index, G, and the CORI values estimated with both M1 and M2. It can be observed that the CORI-G pattern resulting from M2 exhibits a very high value of R<sup>2</sup>, i.e., 92%, indicating the possibility of estimating the corridor operational resilience as a function of the geometric features of a given route.

Finally, M2 is applied to estimate the CORI values of the sample routes for the rainy days during the same period, i.e., September-October 2019, and each route's rainy day-CORI values are plotted in the CORI-G space, as shown in Figure 4.2.7. As can be seen in this figure, the rainy-day CORI-G relationship shows consistently higher CORI values than those of the dry days for the same routes, while maintaining the same structural pattern as the dry-day one, i.e., during rainy days, a freeway corridor system is less resilient than dry days. Further, the high R<sup>2</sup> value of the rainy-day CORI-G pattern indicates the potential applicability of Model 2 to different weather conditions.

Based on the above, in this study, M2 is selected as the enhanced operational-resilience model, which will be applied to the expanded metro freeway network in the subsequent chapter.









0019-09-05

2019-09-25 2019-09-26 0019-10-08

Figure 4.2.3 Comparison of Daily Estimates of CORI from M1 and M2 (Afternoon-Peak Routes)

2019-10-23 2019-10-29 2019-10-30 2019-10-31

2019-10-16

2019-10-03 2019-10-09 2019-10-17 019-10-24

019-09-2

019-09-019-09 019-10-0

2019-09-05 2019-09-11 2019-09-18 2019-09-19

019-09

1-60-6103

2019-09-1











Figure 4.2.5 Comparison of Daily CORI Estimates with Preliminary Model and Enhanced Model (M2)



Figure 4.2.6 CORI vs Geometric Friction Index (G) with Model 1 and Model 2



Figure 4.2.7 Comparison of Dry-Day and Rainy-Day CORI Estimates with Model 2

## 4.3 Summary

This chapter summarized the results from the enhancement and testing of the preliminary model to quantify the operational resilience of freeway corridors. The enhanced model developed in this study considers the time-variant, route-wide space-mean speed as the system performance measure and estimates the resilience of a given route as its capability to resist the route-wide speed reduction and to recover to the free-flow speed in response to the route-wide, mainline volume variations through time. Further, only mainline-traffic data are used in the enhanced model in estimating the operational resilience of a given route, i.e., the enhanced model does not depend on the entrance ramp volumes that may have potential detection issues.

The enhanced model was tested with the same set of data collected from the sample routes used for developing the preliminary model in the previous phase. The test results of the enhanced model showed significantly better performance than those from the preliminary model, i.e., the daily estimates of the operational resilience for the sample routes exhibit consistently less variations than those of the preliminary model. Further, the CORI-Geometric Friction Index (G) relationships for the sample routes have a clear pattern with substantially high correlation, indicating the possibility for developing a functional results of the enhanced model for the rainy days also resulted in the expected CORI-G pattern with strong correlation, i.e., during rainy days, a corridor-traffic system becomes less resilient than dry days.

The enhanced resilience model developed in this chapter is applied in the subsequent chapter to estimate the resilience of the individual corridors in the metro network. Further, the relationships between the resilience estimates and traffic-flow measures of each corridor are also analyzed in the following chapter.

# Chapter 5: ASSESSMENT OF OPERATIONAL RESILIENCE OF INDIVIDUAL CORRIDORS AND THEIR EFFECTS ON TRAFFIC-FLOW PERFORMANCE IN THE METRO NETWORK

# 5.1 Introduction

In this chapter, the corridor-wide operational-resilience model, enhanced in the previous chapter, is applied to the metro freeway network, and the operational resilience of individual directional corridors, both morning and afternoon-peak routes, are estimated with the field traffic data collected over the weekdays of September-October period in 2018, 2019 and 2023. These periods were selected to ensure most freeway corridors in the metro network have experienced certain levels of congestion during peak periods. Further, the relationships between the route-wide geometric configuration and operational resilience are also analyzed in this chapter. The effects of the operational resilience on the traffic-flow performance and the travel-time reliability (TTR) measures at each corridor are analyzed in the subsequent chapter.

# **5.2 Estimation of Operational Resilience of Individual** Directional Corridors in the Metro Network

Figure 5.2.1 shows the configuration of the total 37 individual corridors with 74 directional routes whose corridor-wide operational resilience indices (CORI) were estimated and analyzed in this chapter. First, the daily CORI values of each directional corridor during peak periods for the 2-month period, i.e., from September to October, for 2018, 2019 and 2023 were estimated under dry-weather conditions with the field traffic-detector data. It needs to be noted that, in calculating the daily CORI value of each directional corridor, only those days with at least 40 % of valid detector data were included in this study to minimize the effects of detector-malfunction issues. Next, the average daily CORI value of each directional corridor for the two-month period of each year was determined and shown in Tables 5.2.1 – 5.2.6, which includes the sorted corridor list with the CORI values from low to high for both morning and afternoon routes under dry-weather conditions.



Figure 5.2.1 Configuration of Individual Corridors for the Metro Freeway Network

It needs to be noted that smaller values of CORI indicate higher levels of resilience, i.e., more resilient corridors. Further, the CORI values of all the directional corridors are grouped into 6 color-coded levels as follows:

*Level* 1:  $0 \le CORI < 0.005$  *Level* 2:  $0.005 \le CORI < 0.01$  *Level* 3:  $0.01 \le CORI < 0.015$  *Level* 4:  $0.015 \le CORI < 0.02$  *Level* 5:  $0.02 \le CORI < 0.025$ *Level* 6:  $0.025 \le CORI$ 

Figures 5.2.2 – 5.2.7 show the color-coded CORI level of each directional corridor under dry-weather conditions for both morning and afternoon peak periods in 2018, 2019 and 2023. As noted in those tables and figures, the CORI levels of the individual corridors in the metro freeway network mostly belong to Level 2 and 3 over the selected 3-year period, while a group of certain routes have consistently exhibited higher CORI values, i.e., Level 4 or higher, indicating relatively weaker resilience levels than other corridors. Tables 5.2.7 and 5.2.8 include the list of those individual corridors whose CORI levels have been greater than or equal to Level 4 at least once during the 3-year study period.

#### EFFECTS OF GEOMETRY ON OPERATIONAL RESILIENCE

Finally, the effects of the geometric configuration on the corridor-wide operational resilience are also analyzed in this chapter by examining the relationships between the Geometric Friction (G) Factor, described in the previous chapter, and the CORI values of each directional corridor, both morning and afternoon-peak routes. Figures 5.2.8 – 5.2.13 show the G-CORI plots over the 3-year period. As can be noted in these figures, there exists a clear pattern between the G and CORI values of individual corridors, i.e., as the corridor-G values increase, the CORI values decrease, i.e., the corridors with low geometric friction to through traffic have strong resilience than those with high level of interruption.

Table 5.2.1	CORI	Values	(2018,	AM,	Dry	Days)	

	Corridor	Length (Miles)	Geometric Friction Index (G)	Operational Resilience Index (R)
1	I-94 (EB)_MPLS DT to SP DT	8.4	3.10	0.0020
2	I-35E (NB)_135 to 1494	10.9	2.69	0.0048
3	I-494 (WB)_I94 to I35E	12.4	3.12	0.0049
4	I-694 (WB)_I35E to TH10	3.8	2.84	0.0056
5	T.H.10 (EB)_TH169 to TH610	7.5	1.93	0.0064
6	I-35E (SB)_135 to 1694	13.5	4.14	0.0068
7	I-694 (WB)_TH36 to I35E	6.5	2.37	0.0068
8	I-94 (EB)_I494 to TH252	8	3.63	0.0068
9	I-394 (EB)_I494 to TH169	2.7	1.78	0.0077
10	T.H.52 (NB)_TH55 to SP DT	8.2	2.14	0.0087
11	I-35E (SB)_I694 to SP DT	6	3.41	0.0089
12	I-35W (SB)_I694 to MPLS DT	9.1	3.56	0.0089
13	I-35W (NB)_I494 to MPLS DT	9.2	3.68	0.0093
14	I-94 (WB)_SP DT to MPLS DT	8.5	3.97	0.0094
15	I-494 (WB)_I35W to TH212	5.9	2.42	0.0097
16	T.H.100 (SB)_1694 to 1394	6.8	2.37	0.0099
17	T.H.100 (SB)_1394 to 1494	7.7	2.90	0.0100
18	T.H.610 (EB)_194 to TH10	11.7	3.99	0.0100
19	I-94 (EB) TH252 to MPLS DT	8.1	3.08	0.0102

20		6.1	2.04	0.0104
20	1.H.// (NB)_12/th to 1H62	6.1	2.01	0.0104
21	I-494 (SB)_I694 to TH212	15.5	3.64	0.0106
22	I-35W (NB)_I35 to I494	8.4	3.14	0.0114
23	I-94 (WB)_1694 to SP DT	7.5	2.67	0.0124
24	I-494 (WB) 135E to 135W	7.8	2.70	0.0128
25	I-394 (EB) TH169 to MPLS DT	5.5	3.11	0.0132
26	T.H.10 (EB) TH610 to 1694	7.3	2.80	0.0134
27	I-35W (SB) 135 to 1694	14.8	3,89	0.0134
28	1-694 (NB) 194 to TH36	4.9	1 39	0.0136
20	1-35E (NB) 1494 to SP DT	7.6	3 50	0.0138
30	T H 36 (W/B) 1694 to 135F	6.4	2 34	0.0145
21	L 604 (W/R) TH10 to TH252	5.5	2.54	0.0155
31		10.0	2.80	0.0155
32	0.5.109 (SB)_1H010 (01394	10.9	2.01	0.0167
33	1.H.62 (WB)_TH55 to I35W	4.2	3.10	0.0176
34	T.H.36 (WB)_I35E to I35W	4.4	2.16	0.0177
35	T.H.62 (WB)_135W to 1494	6.7	1.80	0.0202
36	U.S.169 (SB)_I394 to TH13	16.2	2.64	0.0202



Figure 5.2.2 CORI Levels (2018, AM, Dry Days)

## Table 5.2.2 CORI Values (2018, PM, Dry Days)

	Corridor	Length (Miles)	Geometric Friction Index (G)	Operational Resilience Index (R)
1	I-94 (EB)_SP DT to I694	7.3	3.20	0.0044
2	I-94 (WB)_MPLS DT to TH252	8	3.52	0.0051
3	I-94 (WB)_1494 to Rogers	8.4	2.60	0.0056
4	1394 (WB)_TH169 to 1494	3.3	2.99	0.0056
5	T.H.610 (WB)_TH10 to I94	11.3	1.76	0.0063
6	I-35E (SB)_I494 to I35	11	2.30	0.0067
7	I-35W (SB)_I494 to I35	8.6	2.97	0.0069
8	I-494 (EB)_I35W to I35E	7.8	3.58	0.0071
9	I-694 (EB)_TH10 to I35E	4.7	4.33	0.0076
10	T.H.77 (SB)_TH62 to 127th	8.2	2.32	0.0080
11	T.H.10 (WB)_TH610 to TH169	7.6	2.05	0.0081
12	I-494 (NB)_TH212 to I694	14.4	3.15	0.0084
13	1394 (WB)_MPLS DT to TH169	5.6	1.97	0.0090
14	I-94 (EB)_MPLS DT to SP DT	8.4	3.10	0.0090

15	T.H.36 (EB)_I35E to I694	6.6	2.75	0.0092
16	I-35W (NB)_MPLS DT to I694	9.2	2.23	0.0092
17	I-35E (NB)_SP DT to I694	5.8	3.31	0.0093
18	T.H.100 (NB)_1394 to 1694	6.9	2.83	0.0097
19	I-35W (NB)_1694 to 135	14.7	3.64	0.0105
20	I-494 (EB)_I35E to I94	12.4	3.45	0.0108
21	T.H.100 (NB)_I494 to I394	7.6	2.83	0.0120
22	I-694 (EB)_TH252 to TH10	5.4	2.13	0.0122
23	T.H.62 (EB)_I35W to TH55	4.8	1.98	0.0122
24	I-35W (SB)_MPLS DT to I494	9.1	3.45	0.0124
25	I-94 (WB)_SP DT to MPLS DT	8.5	3.97	0.0127
26	I-694 (SB)_TH36 to I94	5.3	2.68	0.0130
27	T.H.10 (WB)_l694 to TH610	7.7	2.92	0.0135
28	I-694 (EB)_I35E to TH36	5.7	1.78	0.0138
29	I-494 (EB)_TH212 to I35W	6.2	2.25	0.0142
30	I-35E (SB)_SP DT to I494	7.5	2.39	0.0144
31	U.S.169 (NB)_I394 to TH610	11.1	2.21	0.0161
32	T.H.36 (EB)_135W to 135E	5.2	1.89	0.0165
33	U.S.169 (NB)_TH13 to I394	16	2.22	0.0185
34	T.H.62 (EB)_1494 to 135W	6.9	1.92	0.0246



Figure 5.2.3 CORI Levels (2018, PM, Dry Days)

## Table 5.2.3 CORI Levels (2018, PM, Dry Days)

	Corridor	Length (Miles)	Geometric Friction Index (G)	Operational Resilience Index (R)
1	I-94 (EB)_MPLS DT to SP DT	8.4	3.10	0.0017
2	I-35E (NB)_135 to 1494	10.9	2.69	0.0052
3	I-394 (EB)_I494 to TH169	2.7	1.78	0.0052
4	I-94 (EB)_1494 to TH252	8	3.63	0.0066
5	I-494 (WB)_I35W to TH212	5.9	2.42	0.0075
6	I-494 (WB)_194 to 135E	12.4	3.12	0.0075
7	I-94 (WB)_SP DT to MPLS DT	8.5	3.97	0.0081
8	T.H.100 (SB) 1394 to 1494	7.7	2.90	0.0087
9	I-494 (SB) 1694 to TH212	15.5	3.64	0.0092
10	I-35E (SB) 135 to 1694	13.5	4.14	0.0093
11	T.H.610 (EB) 194 to TH10	11.7	3.99	0.0094
12	I-694 (WB)_I35E to TH10	3.8	2.84	0.0095

13	I-35E (SB)_I694 to SP DT	6	3.41	0.0095
14	T.H.77 (NB)_127th to TH62	6.1	2.01	0.0098
15	T.H.100 (SB)_1694 to 1394	6.8	2.37	0.0103
16	T.H.10 (EB)_TH169 to TH610	7.5	1.93	0.0105
17	I-35W (NB) 135 to 1494	8.4	3.14	0.0110
18	I-94 (EB) TH252 to MPLS DT	8.1	3.08	0.0111
19	I-35W (NB) 1494 to MPI S DT	9,2	3.68	0.0112
20	1 694 (WR) TH26 to 1255	6.5	2 27	0.0117
20	1-054 (WB)_11130 to 135E	0.5	2.37	0.0117
21	I-394 (EB)_TH169 to MPLS DT	5.5	3.11	0.0118
22	I-35E (NB)_I494 to SP DT	7.6	3.50	0.0119
23	I-694 (NB)_I94 to TH36	4.9	1.39	0.0125
24	I-94 (WB)_I694 to SP DT	7.5	2.67	0.0132
25	I-494 (WB)_I35E to I35W	7.8	2.70	0.0141
26	U.S.169 (SB)_I394 to TH13	16.2	2.64	0.0152
27	I-694 (WB)_TH10 to TH252	5.5	2.80	0.0153
28	T.H.10 (EB)_TH610 to I694	7.3	2.80	0.0154
29	I-35W (SB) 1694 to MPLS DT	9.1	3.56	0.0165
30	U.S.169 (SB) TH610 to I394	10.9	2.01	0.0175
31	T.H.62 (WB) TH55 to 135W	4.2	3.10	0.0177
32	T H 36 (WB) 135E to 135W	4.4	2.16	0.0184
52		4.4	2.10	0.0104
33	T.H.62 (WB)_I35W to I494	6.7	1.80	0.0196
34	T.H.36 (WB)_1694 to 135E	6.4	2.34	0.0231



Figure 5.2.4 CORI Levels (2019, AM, Dry Days)

#### Table 5.2.4 CORI Values (2019, PM, Dry Days)

	Corridor	Length (Miles)	Geometric Friction Index (G)	Operational Resilience Index (R)
1	I-94 (WB)_TH252 to I494	7.1	3.02	0.0050
2	I-35E (SB)_I494 to I35	11	2.30	0.0053
3	I-94 (WB)_MPLS DT to TH252	8	3.52	0.0055
4	I-94 (WB)_I494 to Rogers	8.4	2.60	0.0056
5	I-94 (EB)_SP DT to I694	7.3	3.20	0.0062
6	T.H.610 (WB)_TH10 to I94	11.3	1.76	0.0065
7	T.H.10 (WB)_TH610 to TH169	7.6	2.05	0.0065
8	1394 (WB)_MPLS DT to TH169	5.6	1.97	0.0079
9	I-494 (EB)_I35W to I35E	7.8	3.58	0.0080
10	T.H.36 (EB)_135E to 1694	6.6	2.75	0.0081
11	I-694 (EB)_I35E to TH36	5.7	1.78	0.0090
12	T.H.77 (SB)_TH62 to 127th	8.2	2.32	0.0092
13	I-94 (EB)_MPLS DT to SP DT	8.4	3.10	0.0092
14	I394 (WB)_TH169 to I494	3.3	2.99	0.0095
15	I-35W (SB)_I494 to I35	8.6	2.97	0.0099
16	I-35E (NB)_SP DT to I694	5.8	3.31	0.0099
17	I-35E (NB)_1694 to 135	14	3.16	0.0100
18	I-494 (NB)_TH212 to I694	14.4	3.15	0.0100
19	I-494 (EB)_I35E to I94	12.4	3.45	0.0102
20	I-94 (WB)_SP DT to MPLS DT	8.5	3.97	0.0103
21	T.H.100 (NB)_I394 to I694	6.9	2.83	0.0105
22	I-35W (NB)_MPLS DT to I694	9.2	2.23	0.0110
23	T.H.10 (WB)_I694 to TH610	7.7	2.92	0.0120
24	I-694 (EB)_TH252 to TH10	5.4	2.13	0.0122
25	T.H.100 (NB)_1494 to 1394	7.6	2.83	0.0125
26	I-35E (SB)_SP DT to I494	7.5	2.39	0.0127
27	I-694 (EB)_TH10 to I35E	4.7	4.33	0.0137

28	I-694 (SB)_TH36 to I94	5.3	2.68	0.0146
29	T.H.62 (EB)_135W to TH55	4.8	1.98	0.0148
30	I-35W (SB) MPLS DT to 1494	9.1	3.45	0.0149
31	I-494 (EB) TH212 to I35W	6.2	2.25	0.0150
32	U S 169 (NB) 1394 to TH610	11 1	2 21	0.0175
32	T H 36 (FB) 135W/ to 135F	5.2	1 89	0.0175
24	LLS 160 (NR) TU12 to 1204	10	2.22	0.0211
54	0.3.109 (100] 1013 (01394	10	2.22	0.0211
35	T.H.62 (EB)_I494 to I35W	6.9	1.92	0.0238



Figure 5.2.5 CORI Levels (2019, PM, Dry Days)

Table 5.2.5 CORI Values	(2023, AM, Dry Days)
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	Corridor	Length (Miles)	Geometric Friction Index (G)	Operational Resilience Index (R)
1	I-694 (WB)_I35E to TH10	3.8	2.84	0.0054
2	I-94 (EB)_MPLS DT to SP DT	8.4	3.10	0.0056
3	I-35E (SB)_1694 to SP DT	6	3.41	0.0060

	1	1	1	ı ı
4	I-694 (WB)_TH36 to I35E	6.5	2.37	0.0061
5	I-94 (EB)_I494 to TH252	8	3.63	0.0064
6	I-35W (NB)_I35 to I494	8.4	3.14	0.0068
7	T.H.100 (SB)_1394 to 1494	7.7	2.90	0.0068
8	T.H.100 (SB)_1694 to 1394	6.8	2.37	0.0074
9	I-494 (WB)_I35W to TH212	5.9	2.42	0.0074
10	I-494 (SB)_1694 to TH212	15.5	3.64	0.0078
11	I-94 (EB)_TH252 to MPLS DT	8.1	3.08	0.0082
12	I-94 (WB)_SP DT to MPLS DT	8.5	3.97	0.0084
13	T.H.77 (NB)_127th to TH62	6.1	2.01	0.0084
14	I-35W (SB)_I694 to MPLS DT	9.1	3.56	0.0086
15	I-35W (NB)_I494 to MPLS DT	9.2	3.68	0.0105
16	I-94 (EB)_Rogers to I494	9.1	3.46	0.0116
17	I-35W (SB)_135 to 1694	14.8	3.89	0.0116
18	I-394 (EB)_TH169 to MPLS DT	5.5	3.11	0.0119
19	T.H.10 (EB)_TH610 to I694	7.3	2.80	0.0121
20	U.S.169 (SB)_1394 to TH13	16.2	2.64	0.0123
21	I-94 (WB)_I694 to SP DT	7.5	2.67	0.0124
22	I-694 (NB)_I94 to TH36	4.9	1.39	0.0131
23	I-494 (WB)_I35E to I35W	7.8	2.70	0.0137
24	I-35E (NB)_I494 to SP DT	7.6	3.50	0.0137
25	I-694 (WB)_TH10 to TH252	5.5	2.80	0.0161
26	T.H.36 (WB)_I35E to I35W	4.4	2.16	0.0185
27	U.S.169 (SB)_TH610 to I394	10.9	2.01	0.0205
28	T.H.610 (EB)_194 to TH10	11.7	3.99	0.0220
29	T.H.62 (WB)_TH55 to I35W	4.2	3.10	0.0220
30	T.H.36 (WB)_1694 to 135E	6.4	2.34	0.0242
31	T.H.62 (WB)_I35W to I494	6.7	1.80	0.0267



Figure 5.2.6 CORI Levels (2023, AM, Dry Days)

	Corridor	Length (Miles)	Geometric Friction Index (G)	Operational Resilience Index (R)
1	I-94 (EB)_SP DT to I694	7.3	3.20	0.0037
2	1394 (WB)_MPLS DT to TH169	5.6	1.97	0.0049
3	I-35W (NB)_MPLS DT to I694	9.2	2.23	0.0049
4	I-35W (SB)_I494 to I35	8.6	2.97	0.0055
5	I-494 (EB)_I35W to I35E	7.8	3.58	0.0058
6	I-94 (WB)_MPLS DT to TH252	8	3.52	0.0063
7	T.H.10 (WB)_I694 to TH610	7.7	2.92	0.0066
8	I-35E (NB)_SP DT to I694	5.8	3.31	0.0069
9	I-494 (NB)_TH212 to I694	14.4	3.15	0.0076

## Table 5.2.6 CORI Values (2023, PM, Dry Days)

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10	T.H.10 (WB)_TH610 to TH169	7.6	2.05	0.0078
11	I-35W (SB) MPLS DT to 1494	9.1	3.45	0.0081
		5.1	0110	
12	I-94 (EB)_MPLS DT to SP DT	8.4	3.10	0.0083
13	I-694 (EB)_TH10 to I35E	4.7	4.33	0.0090
14	T.H.36 (EB)_135E to 1694	6.6	2.75	0.0094
15	I-694 (EB)_I35E to TH36	5.7	1.78	0.0101
16	T.H.100 (NB) 1494 to 1394	7.6	2.83	0.0102
17	T.H.100 (NB)_1394 to 1694	6.9	2.83	0.0103
18	I-694 (EB)_TH252 to TH10	5.4	2.13	0.0113
19	I-94 (WB)_SP DT to MPLS DT	8.5	3.97	0.0115
20	U.S.169 (NB)_I394 to TH610	11.1	2.21	0.0121
21	T.H.610 (WB)_TH10 to I94	11.3	1.76	0.0130
22	T.H.62 (EB)_I35W to TH55	4.8	1.98	0.0137
23	U.S.169 (NB) TH13 to I394	16	2.22	0.0137
24	I-35F (SB) SP DT to 1494	7.5	2,39	0.0143
25	I-494 (FB) TH212 to I35W	6.2	2.25	0.0148
		0.2	2.23	0.0140
26	I-94 (WB)_I494 to Rogers	8.4	2.60	0.0153
27	T.H.36 (EB)_I35W to I35E	5.2	1.89	0.0162
28	T.H.62 (EB)_1494 to 135W	6.9	1.92	0.0224


Figure 5.2.7 CORI Levels (2023, PM, Dry Days)

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	Corridor	Length (Miles)	Geometric Friction Index (G)	Operational Resilience Index (R)
1	T.H.36 (WB)_694 to I35E	6.4	2.34	0.0237
2	T.H.62 (WB)_I35W to 494	6.7	1.80	0.0221
3	T.H.610 (EB)_194 to TH10	11.7	3.99	0.0220
4	T.H.62 (WB) TH55 to I35W	4.2	3.10	0.0191
5	U.S.169 (SB) TH610 to 394	10.9	2.01	0.0182
6	T.H.36 (WB)_I35E to I35W	4.4	2.16	0.0182

# Table 5.2.7 Average CORI Values of Individual Corridors with Level 4 or Higher (Morning-Peak Period)

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7	U.S.169 (SB)_394 to 101	16.2	2.64	0.0177
8	I-35W (SB)_694 to MSPL DT	9.1	3.56	0.0165
9	I-694 (WB)_TH10 to I94	5.5	2.80	0.0157
10	T.H.10 (EB)_TH610 to 694	7.3	2.80	0.0154

Table 5.2.8 Average CORI Values of Individual Corridors with Level 4 or Higher (Afternoon-Peak Period)

	Corridor	Length (Miles)	Geometric Friction Index (G)	Operational Resilience Index (R)
1	T.H.62 (EB)_494 to I35W	6.9	1.92	0.0236
2	U.S.169 (NB)_101 to 394	16	2.22	0.0198
3	U.S.169 (NB)_394 to TH610	11.1	2.21	0.0168
4	T.H.36 (EB)_I35W to I35E	5.2	1.89	0.0167
5	I-94 (WB)_1494 to TH610	8.4	2.60	0.0153
6	I-494 (EB)_212 to I35W	6.2	2.25	0.0150



Figure 5.2.8 Route-wide Resilience vs. Geometric Friction Factor (2018, Dry Days, AM-Peak Routes)



Figure 5.2.9 Route-wide Resilience vs. Geometric Friction Factor (2018, Dry Days, PM-Peak Routes)



Figure 5.2.10 Route-wide Resilience vs. Geometric Friction Factor (2019, Dry Days, AM-Peak Routes)



Figure 5.2.11 Route-wide Resilience vs. Geometric Friction Factor (2019, Dry Days, PM-Peak Routes)



Figure 5.2.12 Route-wide Resilience vs. Geometric Friction Factor (2023, Dry Days, AM-Peak Routes)



Figure 5.2.13 Route-wide Resilience vs. Geometric Friction Factor (2023, Dry Days, PM-Peak Routes)

# 5.3 Effects of Operational Resilience on Travel-Time Reliability and Traffic-Flow Measures

In this section, the effects of the operational resilience on the traffic-flow performance and the traveltime TTR Measures at individual directional corridors are analyzed by using the CORI values determined in the previous section. First, for each directional corridor, the yearly estimates of traffic-flow measures, i.e., Vehicle-Miles-Traveled (VMT) and Delayed-Vehicle-Hours (DVH), as well as a set of the travel-time reliability indices are calculated with TeTRES for the 3-year study period of 2018, 2019 and 2023. Next, the relationships between CORI and those traffic-flow measures and travel-time reliability indices are analyzed for each year. The travel-time reliability indices used in this analysis are as follows:

95<sup>th</sup> %-ile Buffer Index (BI) : (95<sup>th</sup> %-ile Travel Time – Average Travel Time) /( Average Travel Time) 95<sup>th</sup> %-ile Planning Index (PI): (95<sup>th</sup> %-ile Travel Time / Free-Flow Travel Time

### EFFECTS OF OPERATIONAL RESILIENCE ON TRAFFIC-FLOW MEASURES

Figures 5.3.1 - 5.3.6 show the relationships between the CORI values and the yearly estimates of VMT/mile for individual directional corridors for the morning and afternoon-peak periods over the 3-year period. In this study, the yearly VMT values of individual directional corridors were normalized by dividing them with the length of each route.

As shown in these figures, there is a clear linear pattern between VMT/mile and CORI. As the CORI value increases, i.e., as the routes become less resilient, the values of VMT/mile also decrease. Further, the VMT/mile-CORI pattern appears to be consistent in both morning and afternoon-peak routes. This indicates the resilience of a directional corridor directly affects the productivity of a given route-traffic system. The above observation is also noted in the relationships between the DVH/mile and CORI, which are shown in Figures 5.3.7 – 5.3.12. As indicated, the routes with smaller CORI values, i.e., the routes with strong resilience, also exhibit smaller values of delayed-vehicle hours per mile than those with weak resilience.

# EFFECTS OF OPERATIONAL RESILIENCE ON TRAVEL-TIME TTR MEASURES

Figures 5.3.13 – 5.3.18 show the CORI-Buffer Index (BI) relationships of the morning and afternoon routes for the 3-year study period. As noted in these figures, there is a clear and consistent pattern between CORI and BI, i.e., as CORI increases, BI also increases. This indicates that the routes with high CORI values, i.e., weak resilience, have larger levels of fluctuations in travel times than those with strong resilience, i.e., low CORI values.

The effects of the operational resilience on the congestion level are also analyzed by examining the relationships between CORI and Planning Index (PI), a ratio of 95<sup>th</sup> percentile travel time to free-flow travel time, at individual directional corridors. Figures 5.3.19 – 5.3.24 show the CORI-PI plots for both morning and afternoon routes during the study period. The CORI-PI relationships noted in these figures exhibit similar patterns shown in the CORI-BI plots, i.e., as CORI increases, PI also increases. This indicates that the routes with low levels of resilience also have high levels of congestion.

As discussed above, the CORI-BI and CORI-PI relationships of the individual directional corridors in the metro freeway network show the expected patterns, i.e., the routes with strong resilience have low levels of travel-time fluctuations and congestion compared to those with weak resilience.



Figure 5.3.1 Route-wide Resilience Index vs VMT/mile (2018, Morning-Peak Routes)



Figure 5.3.2 Route-wide Resilience Index vs VMT/mile (2018, Afternoon-Peak Routes)



Figure 5.3.3 Route-wide Resilience Index vs VMT/mile (2019, Morning-Peak Routes)



Figure 5.3.4 Route-wide Resilience Index vs VMT/mile (2019, Afternoon-Peak Routes)



Figure 5.3.5 Route-wide Resilience Index vs VMT/mile (2023, Morning-Peak Routes)



Figure 5.3.6 Route-wide Resilience Index vs VMT/mile (2023, Afternoon-Peak Routes)



Figure 5.3.7 Route-wide Resilience Index vs DVH/mile (2018, Morning-Peak Routes)



Figure 5.3.8 Route-wide Resilience Index vs DVH/mile (2018, Afternoon-Peak Routes)



Figure 5.3.9 Route-wide Resilience Index vs DVH/mile (2019, Morning-Peak Routes)



Figure 5.3.10 Route-wide Resilience Index vs DVH/mile (2019, Afternoon-Peak Routes)



Figure 5.3.11 Route-wide Resilience Index vs DVH/mile (2023, Morning-Peak Routes)



Figure 5.3.12 Route-wide Resilience Index vs DVH/mile (2023, Afternoon-Peak Routes)



Figure 5.3.13 Route-wide Resilience Index vs Buffer Index (2018, Morning-Peak Routes)



Figure 5.3.14 Route-wide Resilience Index vs Buffer Index (2018, Afternoon-Peak Routes)



Figure 5.3.15 Route-wide Resilience Index vs Buffer Index (2019, Morning-Peak Routes)



Figure 5.3.16 Route-wide Resilience Index vs Buffer Index (2019, Afternoon-Peak Routes



Figure 5.3.17 Route-wide Resilience Index vs Buffer Index (2023, Morning-Peak Routes)



Figure 5.3.18 Route-wide Resilience Index vs Buffer Index (2023, Afternoon-Peak Routes)



Figure 5.3.19 Route-wide Resilience Index vs Planning Index (2018, Morning-Peak Routes)



Figure 5.3.20 Route-wide Resilience Index vs Planning Index (2018, Afternoon-Peak Routes)



Figure 5.3.21 Route-wide Resilience Index vs Planning Index (2019, Morning-Peak Routes)



Figure 5.3.22 Route-wide Resilience Index vs Planning Index (2019, Afternoon-Peak Routes)







Figure 5.3.24 Route-wide Resilience Index vs Planning Index (2023, Afternoon-Peak Routes)

# 5.4 Summary

In this chapter, the resilience model enhanced in the previous chapter was applied to estimate the corridorwide operational resilience indices, CORI, of 74 directional routes using the traffic data from the September-October period in 2018, 2019 and 2023. Based on the estimation results, a group of the directional routes exhibiting consistently low-level of resilience were identified for both morning and afternoon-peak periods. Further, the effects of the geometric configuration on the operational resilience were also analyzed by examining the relationships between the geometric-friction index, G, developed from the previous phase, and the operational resilience of individual routes. The G-CORI plots for the metro freeway corridors resulted in the expected patterns, i.e., the routes with efficient geometric structure in terms of handling through traffic exhibit strong resilience compared with those routes with high levels of geometric friction. Finally, the effects of the operational resilience on the traffic-flow measures and traveltime reliability at individual directional corridors were also analyzed in this chapter. The analysis results indicate that the routes with strong resilience also have better productivity and reliability, e.g., higher VMT/mile with less variability in travel times, than those routes with low levels of resilience. The findings from this chapter could provide the basis for future improvements of freeway corridors in terms of geometric design and operational strategies.

# Chapter 6: CONCLUSIONS – RESEARCH BENEFITS/IMPLEMENTATION/FUTURE STUDY NEEDS

This report summarized the major results from the current study, whose main goal was to assess the traveltime reliability and the operational resilience of the Twin Cities' freeway corridors. First, the various types of traffic and non-traffic data required for TeTRES, the Travel-time Reliability Estimation System, developed in previous studies, were collected from September 2020 to December 2023 and processed to populate the TeTRES database, which was used to estimate the travel-time TTR measures of the individual freeway corridors from January 2018 to December 2023. Furthermore, the prototype resilience model, developed from the previous phase, was enhanced with an expanded data set reflecting different weather conditions from the metro freeway network. Using the enhanced resilience model, the operational-resilience indices of individual freeway corridors were estimated and a group of the directional corridors with low levels of resilience were identified. Furthermore, the analysis of the relationships between geometric configuration and operational resilience resulted in the expected patterns, i.e., the routes with efficient geometric structure for through traffic exhibited strong resilience compared to those routes with high levels of geometric friction. It was also found that the routes with strong resilience had higher VMT/mile with less variability of travel times, i.e., smaller values of buffer and planning indices, than the routes with low levels of resilience. The above findings could provide specific directions for future improvements of freeway corridors in terms of geometric design and operational strategies.

#### RESEARCH BENEFITS

**Decrease Engineering/Administrative Costs:** The updated TeTRES database populated with the new set of historical data, including various types of traffic/non-traffic data, such as weather, incident, work zone, winter-road conditions and special events, as well as the monthly/yearly estimates of the travel-time reliability and traffic-flow measures that resulted from this study for a set of the predefined routes can substantially reduce the time and effort of MnDOT staff in collecting, processing data and analyzing these measures.

**Enhance Effectiveness of Operations and Maintenance:** The results from this study, i.e., the travel-time reliability under different operating conditions, such as incident type and weather, can be directly applicable to MnDOT staff in identifying the major sources of recurring/non-recurring congestion at a corridor level. This can lead to the development of corridor-specific operation and maintenance strategies. Furthermore, the resilience estimates for individual corridors can be applied in prioritizing corridors for operational improvements. Such capability to develop prioritized, corridor-specific improvement plans can substantially improve the efficiency and effectiveness of the MnDOT operational and maintenance strategies for the metro freeway network.

**Environmental Aspects:** The prioritized, corridor-specific improvements in operations and maintenance can contribute to mitigating traffic congestion in the metro freeway network, resulting in reductions in excess energy consumption and emissions due to smoother traffic flow and less queueing.

**Safety:** The assessment of the travel-time reliability at corridor levels under different operating conditions can lead to accurate understanding of the main sources of congestion and travel-time variations. Such an understanding, with the resilience-based prioritization of corridors, can directly lead to proactively improving the operational efficiency and safety of each corridor, thus, reducing the incident-related delays and queueing, which could further increase the risk of secondary crashes.

**Reduce Risk:** The reliability trends and resilience measures can be directly applicable in prioritizing metro freeway corridors for short/long-term improvements, and therefore, can reduce the risk of inefficient allocation of the operational resources for the metro network. Further, it can be pointed out that travel-time variability represents an important source of risk to both passengers and freight traffic. Improving corridor-wide travel-time reliability and resilience could substantially reduce the risks faced by various users.

*Other – Sustainability*: A reliable and resilient freeway network, which reduces the frequency and severity of degraded performance, can improve the environmental and economic sustainability of the Twin Cities, Minnesota, by reducing the external costs of emissions from fuel consumption as well as time and money expenditure of the public.

# IMPLEMENTATION STEPS

To facilitate the realization of the benefits from this study, the following steps will be taken in cooperation with the relevant offices in MnDOT:

- Technical workshops will be conducted for the relevant MnDOT staff regarding the main results from this study and potential applications.
- All the TeTRES files including the updated database will be provided to MnDOT as needed.
- Technical assistance to each MnDOT office will be provided in estimating and analyzing the travel-time reliability and resilience measures for office-specific applications, e.g., before/after performance analysis and corridor prioritization for short/long-term planning.

# FUTURE STUDY NEEDS

Future research needs identified from this study are as follows:

• Continuous collection of data and assessment of travel-time reliability of the freeway corridors in the metro network.

- Enhancement of the geometry-friction model with the explicit incorporation of the connectivity and accessibility of a given route to adjacent corridors.
- Study on the potential effects of heavy-vehicle flows on the corridor-wide operational resilience, since the operational characteristics of heavy vehicles, i.e., acceleration and deceleration, could significantly affect the congestion formulation and recovery patterns of a given route.
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APPENDIX A: Monthly and Yearly Estimates of Travel-Time Reliability and Traffic-Flow Measures for Individual Directional Corridors in the Metro Freeway Network (1/2018 – 12/2023)

# **TABLE OF CONTENTS**

A.1 Configuration of Freeway Corridors for Travel-Time Reliability/Traffic-Flow MOE Estimation		
A.2 Monthly/Yearly Trends of Travel-Time Reliability/Traffic-Flow Measures for Individual Route	A-3	
1) US 169 Corridor A (TH13 < > 394, NB/SB)	A-3	
2) US-169 Corridor B (394 < > TH610, NB/SB)	A-12	
3) TH-610 (TH10 < > I94, WB/EB)	A-21	
4) TH-100 Corridor A (694 < > I394, SB/NB)	A-30	
5) TH-100 Corridor B (I394 < > 494, SB/NB)	A-39	
6) TH-62 Corridor A (494 <> I35W, EB/WB)	A-48	
7) TH-62 Corridor B (I35W < > 55, EB/WB)	A-57	
8) TH-52 (TH55 < > STPL, NB/SB)	A-65	
9) TH-36 Corridor A (694 < > I35E, WB/EB)	A-73	
10) TH-36 Corridor B (I35E < > I35W, WB/EB)	A-82	
11) TH-10 Corridor A (TH169 < > TH610, EB/WB)	A-91	
12) TH-10 Corridor B (TH610 < > I694, EB/WB)	A-100	
13) I-494 Corridor A (I35E to I35W, WB/EB)	A-109	
14) I-494 Corridor B (I35W < > TH212, WB/EB)	A-118	
15) I-494 Corridor C (I35E < > I94, EB/WB)	A-127	
16) I-494 Corridor D (TH212 < > I694, NB/SB)	A-136	
17) I-394 Corridor A (I494 < > TH169, EB/WB)	A-145	
18) I-394 Corridor B (TH169 < > MPLS, EB/WB)	A-154	
19) I-94 Corridor A (Rogers < > I494, EB/WB)	A-163	
20) I-94 Corridor B (I494 < > TH252, EB/WB)	A-172	
21) I-94 Corridor C (TH252 < > MPLS, EB/WB)	A-181	
22) I-94 Corridor D (MPLS < – > STPL, EB/WB)	A-190	

23) I-94 Corridor E (STPL < > I694, EB/WB)	A-207
24) I-35W Corridor A (I35 < > I494, NB/SB)	A-216
25) I-35W Corridor B (I494 < > MPLS, NB/SB)	A-225
26) I-35W Corridor C (MPLS < > I694, NB/SB)	A-234
27) I-35E Corridor A (I35 to I494, NB/SB)	A-243
28) I-35E Corridor B (I494 < > STPL, NB/SB)	A-252
29) I-35E Corridor C (STPL < > I694, NB/SB)	A-261
30) I-35E Corridor D (I694 < > I35, NB/SB)	A-270
31) I-694 Corridor A (I-94 to TH36, NB/SB)	A-279
32) I-694 Corridor B (TH36 < > I-35E, WB/EB)	A-288
33) I-694 Corridor C (TH10 < > TH252, WB/EB)	A-297
34) I-694 Corridor D (TH10 < > I35E, EB/WB)	A-306
35) T.H.77 (127th to TH62, NB/SB)	A-315

A.3 Effects of Special Events on TTR for Directional Routes with Afternoon Peak Periods	A-324
1) TH-52 Corridor Southbound (STPL to TH55)	A-324
2) TH-36 Corridor B (I-35W to I-35E, Eastbound)	A-326
3) I-394 Corridor B (MPLS to 169, Westbound)	A-328
4) I-394 Corridor A (169 to I494, Westbound)	A-330
5) I-94 Corridor C (MPLS to TH252, Westbound)	A-332
6) I-94 Corridor B (TH252 to I494, Westbound)	A-334
7) I-94 Corridor D (MPLS to STPL, Eastbound/Westbound)	A-336
8) I-94 Corridor E (STPL to I694, Eastbound)	A-338
9) I-35W Corridor B (MPLS to I494, Southbound)	A-340
10) I-35W Corridor A (I494 to I35, Southbound)	A-342

11) I-35W Corridor C (MPLS to I694, Northbound)	A-344
12) I-35E Corridor B (STPL to I494, Southbound)	A-346
13) I-35E Corridor A (I494 to I35, Southbound)	A-348
14) I-35E Corridor C (STPL to I694, Northbound)	A-350
15) I-35E Corridor D (I694 to I35, Northbound) 352	A-

#### A.1 Configuration of Freeway Corridors for Travel-Time Reliability/Traffic-Flow MOE Estimation

Figure A.1.1 shows the metro-freeway network consisting with the individual corridors, whose travel-time reliability (TTR) and traffic-flow measures are estimated with TeTRES for each directional route. The TTR measures of each route are based on the travel times calculated every 5-minute interval during non-holiday weekdays, i.e., Tuesdays, Wednesdays, Thursdays. In addition, a set of the commonly used traffic-flow measures were also estimated and presented for each route. The specific measures and operating conditions used in this analysis are as follows:

#### Travel-time Reliability (TTR) Measures

Buffer Index (BI, 95<sup>th</sup> %ile) = (95<sup>th</sup> %ile Travel Time – Average Travel Time)/(Average Travel Time) Planning Index (PI, 95<sup>th</sup> %ile) = 95<sup>th</sup> %ile Travel Time / Free-Flow Travel Time Travel Rate (TR, 95<sup>th</sup> %ile, minutes/mile) = 95<sup>th</sup> %ile Travel Time / Route Length

#### **Operating Conditions**

Weather: All, Dry, Rain, Snow

Incident: All, No-Incident (N), Property Damage Only (PD), Severe/Fatal (INJ, FA)

Work Zone: All, No-WZ (N), Light Impact-WZ (L), Medium-Heavy Effect WZ (M, H)

Special Event: No-Event, Small ( < 20,000 attendees), Medium-Large ( > 20,000 attendees) events

**Peak Periods**: Morning: 6:00 – 9:00 a.m. Afternoon: 3:30 – 6:30 p.m.

#### Traffic-Flow Measures:

VMT (Vehicle-Miles Traveled), VHT (Vehicle-Hours Traveled), DVH (Delayed Vehicle-Hours)



Figure A.1.1 Configuration of Freeway Corridors (36 Corridors, 74 Peak-Directional Routes)

Table A.1.1 Start/End Stations of Each Directional Route

L2 Corridor	L2 Description	L2 St	art L2 En	d Length(Miles)
L35E (NB)	i35 - 494	 \$870	5826	10.9
1-35E (NB)	494 - Saint Pol DT	S826	5619	7.6
1-35E (NB)	Saint Pol DT - 694	S619	S1447	5.8
1-35E (NB)	694 - i35	S1447	S1504	14
L35E (SB)	Saint Pol DT - 494	S644	S858	7 5
1-35E (SB)	404 :25	0044	5050	11
1-35E (SB)	494 - 155	5050	5905	12 5
1-35E (SB)	135 - 694	5 153 1	5 1464	13.5
1-35E (SB)	694 - Saint Poi DT	51464	5644	6
I-35W (NB)	135 - 494	S911	S43	8.4
I-35W (NB)	494 - Mineapolis DT	S43	S566	8.9
I-35W (NB)	Mineapolis DT - 694	S566	S659	9.2
I-35W (NB)	694 - i35	S659	S1561	14.7
I-35W (SB)	Mineapolis DT - 494	S585	S13	8.4
I-35W (SB)	494 - i35	S13	S915	9
I-35W (SB)	i35 - 694	S1567	S686	14.8
I-35W (SB)	694 - Mineapolis DT	S686	S585	9.1
I-394 (EB)	494 - 169	S269	S274	2.7
I-394 (EB)	169 - Mineapolis DT	S274	S290	5.5
1394 (WB)	Mineapolis DT - 169	S289	S340	5.6
1394 (WB)	169 - 494	S340	S346	3.3
I-494 (EB)	212 - 135w	S474	S120	6.2
I-494 (EB)	i35w-i35e	S120	5863	7.8
L494 (EB)	i35e-i94	5863	S1026	12.4
1494 (NB)	TH212 1694	S485	S730	14.4
I-494 (NB)	16212 - 1094	5465	S730	14.4
I-494 (SB)	1694 - TH212	S209	S473	15.5
I-494 (WB)	135e-135w	S864	S119	7.8
I-494 (WB)	i35w-212	S119	S483	5.9
I-494 (WB)	i94-i35e	S1029	S864	12.4
I-694 (EB)	i94 - TH10	S145	S1074	5.4
I-694 (EB)	TH10 - I35w	S1074	S1452	4.7
I-694 (EB)	135w - 36	S1452	S1397	5.7
I-694 (SB)	36 - i94	S1398	S1028	5.3
I-694 (NB)	194 - 36	S1027	S1418	4.9
I-694 (WB)	36 - i35w	S1419	S1459	6.5
I-694 (WB)	135w - TH10	S1461	S1089	3.8
L694 (M/B)	TH10 - 194	S1089	S144	5 5
104 (EB)		S1115	S211	9.1
1-94 (EB)		3115	3211	5.1
1-94 (EB)	Sam P0 D1 - 094	3499	32131	7.5
I-94 (EB)	694 - 95	S2151	\$1358	9.2
I-94 (EB)	1494 - TH252	S211	S170	8
I-94 (EB)	TH252 - Mineapolis DT	S170	S110	8.1
I-94 (EB)	Mineapolis DT - Saint Pol DT	S110	S499	8.4
I-94 (WB)	I494 - TH610	S208	S1112	8.4
I-94 (WB)	Saint Pol DT - Mineapolis DT	S500	S1943	8.3
I-94 (WB)	95 - 694	S1359	S1062	9
I-94 (WB)	694 - Saint Pol DT	S1062	S500	7.5
I-94 (WB)	Mineapolis DT - TH252	S76	S159	8
I-94 (WB)	TH252 - 1494	S159	S216	7.1
T.H.10 (EB)	47 - TH610	S940	S954	7.5
T.H. 10 (EB)	TH610 - 694	S954	S1825	7.3
TH 10 (WB)	694 - TH610	S1820	S978	77
TH 10 (WB)	TH610 - 47	S978	5992	7.6
	494 1394	3976	6392	7.0
T.H. 100 (NB)	1204 604	5375	5391	7.6
T.H. 100 (NB)	1334 - 094	5391	51627	6.9
T.H. 100 (SB)	094 - 1394	\$1615	5405	6.8
T.H.100 (SB)	1394 - 494	S405	S421	7.7
T.H.36 (EB)	135w - 135e	S587	S599	5.2
T.H.36 (EB)	135e - 694	S600	S1427	6.6
T.H.36 (WB)	135e - 135w	S609	S618	4.4
T.H.36 (WB)	694 - I35e	S1425	S608	11.2
T.H.52 (NB)	TH55 to Saint Paul Downtown	S1166	S1178	8.2
T.H.52 (SB)	Saint Paul Downtown to TH55	S1151	S1163	8.4
T.H.610 (EB)	194 to TH10	S1954	S966	8.2
T.H.610 (WB)	TH10 to 194	S996	S1961	8.4
T.H.62 (EB)	494 - 135w	S301	S69	69
TH 62 (EB)	135w - 55	560	S1135	18
TH 62 (MR)	135w - 494		<u> </u>	4.0
	55 I35w	5127	S309	0.7
	101 204	5330	5127	4.2
0.5.109 (NB)	101 - 394 2014 - TUC10	51617	5441	16
0.S.169 (NB)		S441	S1799	111.1
U.S.169 (SB)	1H6T0 - 394	\$1795	S448	10.9
U.S.169 (SB)	394 - 101	S448	S1626	16.2
T.H.77 (NB)	TH13 to TH62	S799	\$531	6.1
T H 77 (SB)	TH62 to TH13	\$534	5814	82

A.2 Monthly and Yearly Trends of TT-Reliability and Traffic-Flow Measures for Individual Corridors

## 1) US-169 Corridor A (TH 101 < -- > I-394)



i) US-169 Corridor A- NB Route TH 101 to I-394 for Afternoon Peak Period

Location of 169 NB Route

*Effects of Weather conditions on Travel-Time Reliability* 



**Effects of Incidents** 



#### Effects of Work Zones



Yearly Variations of TTR - Weather Effects



**Incident Effects** 



## Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



#### ii) US-169 Corridor A - SB Route I-394 to TH 101 for Moring-Peak Period

#### Effects of Weather conditions on Travel-Time Reliability



Effects of Incidents



#### Effects of Work Zones



Yearly Variations - Weather Effects



**Incident Effects** 



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



## 2) US-169 Corridor B (I394 < -- > TH610)

i) US-169 Corridor B – NB I394 to TH610 for Afternoon Peak Periods



Location of 169 NB Route

Effects of Weather conditions on Travel-Time Reliability



**Effects of Incidents** 



#### Effects of Work Zones



Yearly Variations of TTR - Weather Effects



Incident Effects



## Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



### ii) US-169 Corridor B – SB TH610 to I394 for Morning Peak Periods

#### Effects of Weather conditions on Travel-Time Reliability



Effects of Incidents



**Effects of Work Zones** 



#### Yearly Variations - Weather Effects



### Incident Effects



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



3) TH-610 Corridor (194 < -- > TH10)



i) TH-610 WB Corridor WB TH10 – I94 for Afternoon Peak Period

Effects of Weather conditions on Travel-Time Reliability



**Effects of Incidents** 



#### Effects of Work Zones



## Yearly Variations - Weather Effects



Incident Effects






Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



## ii) TH-610 EB Corridor – EB I94 to TH10 for Morning Peak Periods

#### Effects of Weather conditions on Travel-Time Reliability



**Effects of Incidents** 



#### **Effects of Work Zones**



# Yearly Variations - Weather Effects



Incident Effects



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



# 4) TH-100 Corridor A (1694 < -- > 1394)





*Effects of Weather conditions on Travel-Time Reliability* 



**Effects of Incidents** 



Effects of Work Zones



#### Yearly Variations - Weather Effects



**Incident Effects** 







Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



### ii) TH-100 NB Corridor A- NB I394 to 694 for Afternoon Peak Periods



## Effects of Weather conditions on Travel-Time Reliability

Effects of Incidents



Effects of Work Zones



Yearly Variations - Weather Effects



Incident Effects









# Yearly Variations of Buffer Index - Travel Rate Relationships

Variations of Traffic-Flow Measures



5) TH-100 Corridor B (1394 < -- > 1494)

i) TH-100 Corridor B -SB I394 to I494 for Morning Peak Periods



Effects of Weather conditions on Travel-Time Reliability



### **Effects of Incidents**



Effects of Work Zones



Yearly Variations - Weather Effects



**Incident Effects** 



# Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



## ii) TH-100 Corridor B - NB 494 to I394 for Afternoon Peak Periods



#### Effects of Weather conditions on Travel-Time Reliability

**Effects of Incidents** 



Effects of Work Zones



Yearly Variations - Weather Effects



Incident Effects



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



6) TH-62 Corridor A (1494 <--> 135W)



i) TH-62 Corridor A EB – 1494 to 135W for Afternoon Peak Periods

Effects of Weather conditions on Travel-Time Reliability



Effects of Incidents



Effects of Work Zones


## Yearly Variations - Weather Effects



Incident Effects



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



#### ii) TH-62 Corridor A- WB I35W to 494 for Morning Peak Periods



Effects of Work Zones



Yearly Variations - Weather Effects



Incident Effects



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



7) TH-62 Corridor B (I35W < -- >TH55)



i) TH-62 Corridor B – EB I35W to TH55 for Afternoon Peak Periods



# Effects of Incidents



# Effects of Work Zones



## Yearly Variations - Weather Effects



Incident Effects









# Yearly Variations of Buffer Index - Travel Rate Relationships

Variations of Traffic-Flow Measures



ii) TH-62 Corridor B - WB TH55 to I35W for Morning Peak Periods



Effects of Incidents



## Effects of Work Zones



Yearly Variations - Weather Effects



Incident Effects



## Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



8) TH-52 (TH55 < -- > STPL)



i) TH-52 NB TH55 to STPL for Morning Peak Periods

#### Effects of Weather conditions on Travel-Time Reliability



# Effects of Incidents



Effects of Work Zones



Yearly Variations - Weather Effects



Incident Effects



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



#### ii) TH-52 SB – STPL to TH55 for Afternoon Peak Periods



#### Effects of Weather conditions on Travel-Time Reliability

#### Effects of Incidents



#### Effects of Work Zones



Yearly Variations - Weather Effects



**Incident Effects** 



Work-Zone Effects





# Yearly Variations of Buffer Index - Travel Rate Relationships

Variations of Traffic-Flow Measures



9) TH-36 Corridor A (1694 < -- > 135E)



i) TH-36 Corridor A- WB I694 to I35E for Morning Peak Periods



Effects of Incidents



Effects of Work Zones



## Yearly Variations - Weather Effects



Incident Effects



Work-Zone Effects





Yearly Variations of Buffer Index - Travel Rate Relationships

Variations of Traffic-Flow Measures


### ii) TH-36 Corridor A - EB I35E to 694 for Afternoon Peak Periods



#### Effects of Weather conditions on Travel-Time Reliability

Effects of Incidents



#### Effects of Work Zones



Yearly Variations - Weather Effects



**Incident Effects** 





Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships

Variations of Traffic-Flow Measures



10) TH-36 Corridor B (I35E < -- > I35W)



i) TH-36 Corridor B - WB I35E to I35W for Morning Peak Periods

Effects of Weather conditions on Travel-Time Reliability



Effects of Incidents



Effects of Work Zones





Incident Effects







Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



## ii) TH-36 Corridor B - EB I35W to I35E for Afternoon Peak Period



#### Effects of Weather conditions on Travel-Time Reliability

Effects of Incidents



Effects of Work Zones



Yearly Variations - Weather Effects



**Incident Effects** 



Work-Zone Effects





# Yearly Variations of Buffer Index - Travel Rate Relationships

Variations of Traffic-Flow Measures



11) TH-10 Corridor A (TH169 < -- > TH610)

i) TH-10 Corridor A - EB TH169 to TH610 for Morning Peak Periods



Effects of Weather conditions on Travel-Time Reliability



Effects of Incidents



### Effects of Work Zones





Incident Effects



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



# ii) TH-10 Corridor A - WB TH610 to TH169 for Afternoon Peak Periods



#### Effects of Weather conditions on Travel-Time Reliability

Effects of Incidents



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Incident Effects





#### Work-Zone Effects



# Yearly Variations of Buffer Index - Travel Rate Relationships

Variations of Traffic-Flow Measures



12) TH-10 Corridor B ( TH610 < -- > I694)

i) TH-10 Corridor B -- EB TH610 to I694 for Morning Peak Periods



Effects of Weather conditions on Travel-Time Reliability



Effects of Incidents



Effects of Work Zones





Incident Effects



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures


## ii) TH-10 Corridor B - WB I694 to TH 610 for Afternoon Peak Periods



#### Effects of Weather conditions on Travel-Time Reliability

Effects of Incidents



Effects of Work Zones



Yearly Variations - Weather Effects



**Incident Effects** 



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



# 13) I-494 Corridor A (I35E to I35W)

i) I-494 Corridor A - WB for I-35e -> I-35w, Morning Peak Periods



Effects of Weather conditions on Travel-Time Reliability



Effects of Incidents



#### Effects of Work Zones





Incident Effects





Work-Zone Effects

Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



## ii) I-494 Corridor A - EB I-35w to I-35E for Afternoon Peak Periods

### Effects of Weather conditions on Travel-Time Reliability



Effects of Incidents



#### Effects of Work Zones





Incident Effects



Work-Zone Effects





Yearly Variations of Buffer Index - Travel Rate Relationships

Variations of Traffic-Flow Measures



14) I-494 Corridor B (I35W < -- > TH212)



i) I-494 Corridor B - WB (I-35W to TH 212) for Morning Peak Periods

Effects of Weather conditions on Travel-Time Reliability



Effects of Incidents



Effects of Work Zones





**Incident Effects** 



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



## ii) I-494 Corridor B - EB TH 212 to I-35w for Afternoon Peak Period



#### Effects of Weather conditions on Travel-Time Reliability

Effects of Incidents



### Effects of Work Zones





Incident Effects





Work-Zone Effects



# Yearly Variations of Buffer Index - Travel Rate Relationships

Variations of Traffic-Flow Measures



15) I-494 Corridor C (I35E < -- > I94)



i) I-494 Corridor C - EB I-35E to I-94 for Afternoon Peak Periods

Effects of Weather conditions on Travel-Time Reliability



Effects of Incidents



Effects of Work Zones





**Incident Effects** 



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures


# ii) I-494 Corridor C - WB I-94 to I-35E for Morning Peak Periods



### Effects of Weather conditions on Travel-Time Reliability

Effects of Incidents



Effects of Work Zones



Yearly Variations - Weather Effects



Incident Effects



Work-Zone Effects





Variations of Traffic-Flow Measures



16) I-494 Corridor D (TH212 < -- > I694)



i) I-494 Corridor D - NB TH 212 to I-694 for Afternoon Peak Periods

Effects of Weather conditions on Travel-Time Reliability



Effects of Incidents



#### Effects of Work Zones





**Incident Effects** 





Work-Zone Effects

Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



## ii) I-494 Corridor D - SB I-694 to TH 212 for Morning Peak Periods



#### Effects of Weather conditions on Travel-Time Reliability

Effects of Incidents



Effects of Work Zones



Yearly Variations - Weather Effects



**Incident Effects** 



Work-Zone Effects





Variations of Traffic-Flow Measures





17) I-394 Corridor A (I494 < -- > TH169)

i) I-394 Corridor A - EB I494 to TH169 for Morning Peak Periods



Effects of Weather conditions on Travel-Time Reliability



Effects of Incidents



### Effects of Work Zones





**Incident Effects** 



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



#### ii) I-394 Corridor A - WB TH169 to I494 for Afternoon Peak Periods





Effects of Incidents



Effects of Work Zones





**Incident Effects** 









Variations of Traffic-Flow Measures





# 18) I-394 Corridor B (TH169 < -- > MPLS)

i) I-394 Corridor B – EB TH169 to Minneapolis DT for Morning Peak Periods



Effects of Weather conditions on Travel-Time Reliability



Effects of Incidents



### Effects of Work Zones





Incident Effects





Work-Zone Effects





Variations of Traffic-Flow Measures


## ii) I-394 Corridor B - WB Minneapolis DT to TH169 for Afternoon Peak Periods



### Effects of Weather conditions on Travel-Time Reliability

Effects of Incidents







Incident Effects



Work-Zone Effects





Variations of Traffic-Flow Measures





19) I-94 Corridor A (Rogers < -- > 1494)



*i) I-94 Corridor A - EB Rogers to I494 for Morning Peak Periods Effects of Weather conditions on Travel-Time Reliability* 



Effects of Incidents



Effects of Work Zones





Incident Effects





#### Work-Zone Effects

Yearly Variations of Buffer Index - Travel Rate Relationships

A-222



Variations of Traffic-Flow Measures



# ii) I-94 Corridor A - WB I-494 to Rogers for Afternoon Peak Periods



#### Effects of Weather conditions on Travel-Time Reliability

Effects of Incidents







Incident Effects



Work-Zone Effects





Variations of Traffic-Flow Measures



20) I-94 Corridor B (I494 < -- > TH252)

i) I-94 Corridor B -- EB -I-494 to TH252 for Morning Peak Periods



Effects of Weather conditions on Travel-Time Reliability



Effects of Incidents







Incident Effects







Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



# ii) I-94 Corridor B -- WB TH252 to I494 for Afternoon Peak Periods

# Effects of Weather conditions on Travel-Time Reliability



Effects of Incidents



Effects of Work Zones





Incident Effects



Work-Zone Effects





Variations of Traffic-Flow Measures



21) I-94 Corridor C (TH252 < -- > MPLS)

i) I-94 Corridor C -- EB TH252 to Minneapolis DT for Morning Peak Periods



Effects of Weather conditions on Travel-Time Reliability



Effects of Incidents







**Incident Effects** 









Variations of Traffic-Flow Measures


# ii) I-94 Corridor C -- WB MPLS DT to TH252 for Afternoon Peak Periods



### Effects of Weather conditions on Travel-Time Reliability

Effects of Incidents



### Effects of Work Zones



# Yearly Variations - Weather Effects



Incident Effects



Work-Zone Effects





Variations of Traffic-Flow Measures



22) I-94 Corridor D (MPLS < - > STPL)



*i) I-94 Corridor D -- EB MPLS to STPL for Afternoon Peak Periods* 

Effects of Weather conditions on Travel-Time Reliability



**Effects of Incidents** 



## Effects of Work Zones



# Yearly Variations - Weather Effects



Incident Effects









Variations of Traffic-Flow Measures





## ii) I-94 Corridor D-- EB MPLS to STPL for Morning Peak Periods



## Effects of Weather conditions on Travel-Time Reliability

Effects of Incidents



## Effects of Work Zones



Yearly Variations - Weather Effects



Incident Effects





#### Work-Zone Effects



Variations of Traffic-Flow Measures



iii) I-94 Corridor D -- WB STPL to MPLS for Morning Peak Periods



#### Effects of Weather conditions on Travel-Time Reliability

Effects of Incidents



### Effects of Work Zones



Yearly Variations - Weather Effects



Incident Effects





#### Work-Zone Effects



Variations of Traffic-Flow Measures



iv) I-94 Corridor D -- WB STPL to MPLS to Afternoon Peak Periods



Effects of Weather conditions on Travel-Time Reliability

Effects of Incidents



### Effects of Work Zones



# Yearly Variations - Weather Effects



Incident Effects



Work-Zone Effects





Variations of Traffic-Flow Measures



23) I-94 Corridor E (STPL < -- > 1694)

i) I-94 Corridor E -- EB STPL to I694 for Afternoon Peak Periods



Effects of Weather conditions on Travel-Time Reliability



Effects of Incidents



Effects of Work Zones



# Yearly Variations - Weather Effects



Incident Effects





Work-Zone Effects



Variations of Traffic-Flow Measures



A-281

ii) I-94 Corridor E -- WB 1694 to STPL for Morning Peak Periods

Effects of Weather conditions on Travel-Time Reliability



## Effects of Incidents



Effects of Work Zones


Yearly Variations - Weather Effects



## Incident Effects



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



A-287

# 24) I-35W Corridor A (135 < -- > 1494)



i) I-35W Corridor A -- NB I35 to I494 for Morning Peak Periods

Effects of Weather conditions on Travel-Time Reliability



Effects of Incidents



Effects of Work Zones



Yearly Variations - Weather Effects



**Incident Effects** 



## Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



### ii) I-35W Corridor A -- SB I494 to I35 for Afternoon Peak Periods



#### Effects of Weather conditions on Travel-Time Reliability

Effects of Incidents



#### Effects of Work Zones



Yearly Variations - Weather Effects



Incident Effects





Work-Zone Effects



# Yearly Variations of Buffer Index - Travel Rate Relationships

Variations of Traffic-Flow Measures



25) I-35W Corridor B (1494 < -- > MPLS)



*i) I-35W Corridor B -- NB I494 to MPLS for Morning Peak Period Effects of Weather conditions on Travel-Time Reliability* 



Effects of Incidents



Effects of Work Zones



Yearly Variations - Weather Effects



**Incident Effects** 





Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships

Variations of Traffic-Flow Measures





# ii) I-35W Corridor B SB Route 1 for L2 - MPLS to I494, Afternoon Peak Periods



### Effects of Weather conditions on Travel-Time Reliability

Effects of Incidents



#### Effects of Work Zones



Yearly Variations - Weather Effects



Incident Effects



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



26) I-35W Corridor C (MPLS < -- > 1694)



i) I-35W Corridor C -- NB MPLS to I694 for Afternoon Peak Periods

Effects of Weather conditions on Travel-Time Reliability



Effects of Incidents







# Yearly Variations - Weather Effects



Incident Effects





Work-Zone Effects

Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures


### ii) I-35W Corridor C -- SB I694 to MPLS for Morning Peak Periods



Effects of Incidents



### Effects of Work Zones





Incident Effects



Work-Zone Effects





Variations of Traffic-Flow Measures



27) I-35E Corridor A (135 to 1494)



i) I-35E Corridor A -- NB I35 to I494 for Morning Peak Periods



Effects of Incidents



#### Effects of Work Zones





**Incident Effects** 



Work-Zone Effects





Variations of Traffic-Flow Measures



ii) I-35E Corridor A -- SB I494 to I35 for Afternoon Peak Periods



#### Effects of Incidents



Effects of Work Zones



Yearly Variations - Weather Effects



### Incident Effects



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



28) I-35E Corridor B (1494 < -- > STPL)



i) I-35E Corridor B -- NB I494 to STPL for Morning Peak Periods



Effects of Incidents



#### Effects of Work Zones





Incident Effects



Work-Zone Effects





Variations of Traffic-Flow Measures





### ii) I-35E Corridor B -- SB STPL to I494 for Afternoon Peak Periods



Effects of Incidents









**Incident Effects** 



Work-Zone Effects





Variations of Traffic-Flow Measures



29) I-35E Corridor C (STPL < -- > 1694)



i) I-35E Corridor C -- NB STPL to I694 for Afternoon Peak Periods



Effects of Incidents



#### Effects of Work Zones





**Incident Effects** 



Work-Zone Effects





Variations of Traffic-Flow Measures



*ii) I-35E Corridor C -- SB I694 to STPL for Morning Peak Periods Effects of Weather conditions on Travel-Time Reliability*




Effects of Work Zones



Yearly Variations - Weather Effects



Incident Effects



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



30) I-35E Corridor D ( 1694 < -- > 135)



i) I-35E Corridor D -- NB I694 to I35 for Afternoon Peak Periods





## Effects of Work Zones



Yearly Variations - Weather Effects





Incident Effects

Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures





# ii) I-35E Corridor D -- SB I35 to I694 for Morning Peak Periods



### Effects of Weather conditions on Travel-Time Reliability



#### Effects of Work Zones



Yearly Variations - Weather Effects



**Incident Effects** 









# Yearly Variations of Buffer Index - Travel Rate Relationships

Variations of Traffic-Flow Measures



31) I-694 Corridor A (I-94 to TH36)



i) I-694 Corridor A -- NB I-94 to TH36 for Morning Peak Periods



Effects of Incidents



Effects of Work Zones



#### Yearly Variations - Weather Effects



Incident Effects





Work-Zone Effects

Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures



ii) I-694 Corridor A -- SB TH36 to I94 for Afternoon Peak Periods





Effects of Work Zones



Yearly Variations - Weather Effects







Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships









32) I-694 Corridor B (TH36 < -- > I-35E)



i) I-694 Corridor B -- WB TH36 to I-35E for Morning Peak Periods



Effects of Incidents



Effects of Work Zones



# Yearly Variations - Weather Effects



**Incident Effects** 



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures




# ii) I-694 Corridor B -- EB I35E to TH36 for Afternoon Peak Periods

#### Buffer Index (All) Buffer Index (Dry) Buffer Index (Rain) Buffer Index (Snow) 15 1.5 linit It di bl 2018-01 2018-05 2018-05 2018-05 2018-05 2018-01 2018-01 2018-01 2019-05 2019-05 2019-05 2019-05 2019-05 2019-05 2019-05 2019-05 2019-05 2019-05 2019-05 2019-05 2019-05 2019-05 2019-05 2018-01 2018-1 1000001000 Planning Index (All) Planning Index (Dry) Planning Index (Rain) Planning Index (Snow) 2014601 2014605 201405 2014005 2014005 2014005 2005005 20050000000000000 2018-01 2018-05 2018-05 2018-05 2018-01 2018-01 2018-01 2018-01 2018-01 2018-01 2019-0 2018-01 2018-02 2018-02 2018-02 2018-02 2018-02 2018-02 2018-02 2018-02 2018-02 2018-02 2018-02 2018-02 2019-02 200-02 200-02 200-02 200-02 200-02 200-02 200-02 200-02 200-00 2018-05 2018-05 2018-05 2018-07 2018-07 2018-07 2018-07 2018-03 2019-03 2019-03 2019-05 Travel Rate (All) Travel Rate (Dry) Travel Rate (Rain) Travel Rate (Snow) 2018.03 2018.03 2018.03 2018.07 2018.07 2018.07 2019.07 2019.01 2019.0

Effects of Incidents







**Incident Effects** 



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships







Variations of Traffic-Flow Measures



33) I-694 Corridor C (TH10 < -- > TH252)



i) I-694 Corridor C -- WB TH 10 to TH252 for Morning Peak Periods



**Effects of Incidents** 







Incident Effects



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures





# ii) I-694 Corridor C -- EB TH252 to TH 10 for Afternoon Peak Periods

#### Buffer Index (All) Buffer Index (Dry) Buffer Index (Rain) Buffer Index (Snow) 1.5 15 0.5 1.1 110 2018-01 2019-02 200-02 200-02 200-02 200-02 200-02 200-02 200-02 200-0 2016-010 2018-000 2018-000 2018-000 2018-000 2018-000 2018-000 2018-000 2018-000 2018-000 2018-000 2018-000 2018-000 2018-000 2018-000 2018-000 200 2014-01 2014-05 2000-05 2000-0 Planning Index (All) Planning Index (Dry) Planning Index (Rain) Planning Index (Snow) 20118-01 20138-03 20138-03 20138-03 20138-01 20138-01 20138-01 20138-01 20138-01 20138-01 20138-01 2013-01 200 20118-01 2014-09 2014-09 2014-09 2014-09 2014-09 2014-09 2019-09 2019-09 2019-09 2019-09 2019-09 2019-09 2019-09 2019-01 2019-01 2019-01 2019-01 2019-01 2019-01 2011-09 2000-00 2000-00 2000-00 2000-00 2000-00 2000-00 2000-00 2000-00 2000-00 2000-00 2000-00 20000 199999999 Travel Rate (All) Travel Rate (Dry) Travel Rate (Rain) Travel Rate (Snow)

Effects of Incidents



Effects of Work Zones



Yearly Variations - Weather Effects



Incident Effects



Work-Zone Effects





# Yearly Variations of Buffer Index - Travel Rate Relationships

Variations of Traffic-Flow Measures





34) I-694 Corridor D (TH10 < -- > I35E)



i) I-694 Corridor D -- WB I35E to TH10 for Morning Peak Periods



Effects of Incidents







Incident Effects





#### Work-Zone Effects

Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures





# ii I-694 Corridor D -- EB TH10 to I35E for Afternoon Peak Periods



#### Effects of Weather conditions on Travel-Time Reliability

Effects of Incidents



**Effects of Work Zones** 



Yearly Variations - Weather Effects



Incident Effects



Work-Zone Effects





Yearly Variations of Buffer Index - Travel Rate Relationships

Variations of Traffic-Flow Measures



35) T.H.77 (127th to TH62)



i) T.H 77 NB 127th to TH62 for Morning Peak Periods



Effects of Incidents







Incident Effects



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures





ii) T.H 77 SB TH62 to 127th for Afternoon Peak Periods


Effects of Incidents



Effects of Work Zones



Yearly Variations - Weather Effects



### Incident Effects



Work-Zone Effects



Yearly Variations of Buffer Index - Travel Rate Relationships



Variations of Traffic-Flow Measures





### A.3. Effects of Special Events on TTR for Directional Routes with Afternoon Peak Periods

This section presents the monthly and yearly TTR variations of the individual routes, whose TTR values are affected by the special events during the afternoon peak periods. In this study, the TTR values under the following 'special event' conditions are estimated using TeTRES: '*Near* (within 3 miles of a given route)', '*No Event'*, 'Small Event (Attendance < 20,000)' and 'Medium/Large Event (Attendance > 20,000)'.



### 1) TH-52 Corridor Southbound (STPL to TH55)

i) TH-52 SB Corridor (STPL to TH55)



Yearly variations



2) TH-36 Corridor B (I-35W to I-35E, Eastbound)



i) TH-36 Corridor B -- EB for I-35W to I-35E



Yearly variations



3) I-394 Corridor B (MPLS to US169, Westbound)



i) I-394 Corridor B-- WB for MPLS to US169





# 4) I-394 Corridor A (169 to I494, Westbound)



i) I-394 Corridor A -- WB for L2 - 169 to 1494



Yearly variations



## 5) I-94 Corridor C (MPLS to TH252, Westbound)



i) I-94 Corridor C -- WB for L2 - MPLS to TH252







# 6) I-94 Corridor B (TH252 to I494, Westbound)



i)I-94 Corridor B -- WB for L2 - TH252 to I494





## 7) I-94 Corridor D (MPLS <-> STPL, Eastbound/Westbound)



i) I-94 Corridor D -- EB for MPLS to STPL







*ii) I-94 Corridor D -- WB for STPL to MPLS* 



Yearly variations



### 8) I-94 Corridor E (STPL to I694, Eastbound)



i) I-94 Corridor E -- EB for STPL to I694





9) I-35W Corridor B (MPLS to I494, Southbound)



i) I-35W Corridor B -- SB for MPLS to I494





# 10) I-35W Corridor A (1494 to 135, Southbound)



*i) I-35W Corridor A-- SB for 1494 to 135* 



Yearly variations



#### 11) I-35W Corridor C (MPLS to I694, Northbound)



i) I-35W Corridor C -- NB MPLS to I694





## 12) I-35E Corridor B (STPL to I494, Southbound)



i) I-35E Corridor B -- SB for STPL to I494


Yearly variations



## 13) I-35E Corridor A (1494 to 135, Southbound)



*i) I-35E Corridor A -- SB for 1494 to 135* 

Monthly variations



Yearly variations







*i) I-35E Corridor C NB for STPL to 1694* 

Monthly variations



Yearly variations



15) I-35E Corridor D (1694 to 135, Northbound)



i) I-35E Corridor D -- NB 1694 to 135

Monthly variations



Yearly variations

